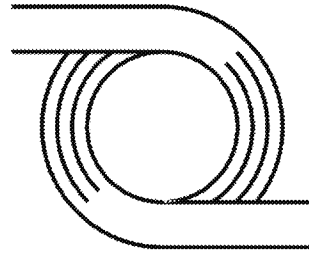


AECOM

**WILSON
MEANY**



D & D ENGINEERING, INC.

PROJECT PRELIMINARY HYDROLOGY REPORT

August 23, 2018

D & D Engineering, Inc.
8901 S. La Cienega Blvd.
Inglewood, CA 90301
424-351-6800



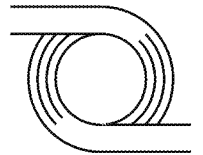


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VI. Post Development runoff 5

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Figure 2 — Existing Storm Drains

Figure 3 — Pre-Development Hydrology Map

Figure 4 — Post-Development Hydrology Map

Figure 5— Proposed Onsite Runoff Distribution to Existing Storm Drain Lines

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Off-Site Hydrology and LACDOW Information Request Summary

- ∞ *Figure 2 — Existing Storm Drains*
- ∞ *Offsite Tributary Area Map for Project 4402 & 681*
- ∞ *Offsite Tributary Area Map for Project DDI #8*
- ∞ *Information Request Summary*



Appendix B

- *LACDPW, 2006 Hydrology Manual, Appendix B Hydrologic Maps, 08 Inglewood*

Appendix C

Pre-Development Runoff Calculations

- ∞ *Figure 3 — Pre-Development Hydrology Map*
- ∞ *Sub-areas HydroCalc Worksheets*

Appendix D

Post-Development Runoff Calculations and Basin Routing

- ∞ *Figure 4 — Post-Development Hydrology Map*
- ∞ *Figure 5 — Proposed Onsite Runoff Distribution to Existing Storm Drain Lines*
- ∞ *Sub-Areas HydroCalc Worksheets*
- ∞ *Summary of Post-Development Runoff Distribution to Existing Storm Drain Systems*

Appendix E

Los Angeles County As-Built Plans

- ∞ *LACDPW Project 4402 & 681 As-Built Plans*
- ∞ *LACDPW Project 4401, Line A As-Built Plans*
- ∞ *LACDPW DDI # 8 As-Built Plans*

I. INTRODUCTION

The purpose of this report is to outline and describe the onsite hydrology and existing storm drain infrastructure serving the existing site where Project Condor is to be constructed on. Additionally, this report will present the on-site hydrology of the proposed project, quantify the proposed storm runoff flows, the proposed, and the new storm drain infrastructure necessary to accommodate the proposed project. New infrastructure design is to adhere to the approved Los Angeles County Department of Public Works (LACDPW) allowable storm drain discharges from the site.

II. SITE DESCRIPTION

Project Condor is comprised of three sites located near the intersection of Century Boulevard and Prairie Avenue in the city of Inglewood. The first and main project site is located to the southeast of the intersection, the second is located to the southwest of the intersection, and the third site is further east, just east of Doty Avenue. The first site of the proposed development includes a multi-purpose sport arena, a parking structure and other miscellaneous use buildings. The site is located on an approximately 17-acre parcel bound by Century Blvd on the North, Prairie Ave on the West, Doty Ave on the East and 103th Street on the South. The second site includes proposed parking structure over an approximately 5.5-acre site, just west of Prairie Avenue. The third site includes proposed approximately 2.9-acre surface parking and portion of vacant land over an approximately 5-acre site, not contiguous to the main project site, just east of Doty Avenue. Refer to the project site map, *Figure 1 — Vicinity Map*, for project site locations.

The existing site over the proposed main project site currently contains commercial buildings, a hotel, a fast-food restaurant and vacant land for significant portion. The existing site over the proposed parking structure site consists of twenty-seven parcels that are all currently vacant lots. The existing site over the proposed surface parking site consists of five parcels that are all currently vacant lots.

The site topography generally slopes from the north-east to south-west corner.

Currently, the existing site runoff is discharging to surrounding public streets where it is collected by the existing storm drain system. There is currently no existing on-site storm drain system in place. The existing runoff breakdown from project sites to the existing storm drain systems is as follows:

- ∞ Majority of main site runoff of 18.4 cfs to Project 4402 and Project 681 County storm drain lines.

- ∞ Parking structure site runoff is split to two systems – portion north of 101st Street runoff of 2.8 cfs to Project 4402 Line B and portion south of 101st Street runoff of 4.8 cfs to DDI#0008 Freeman Avenue County storm drain lines.
- ∞ Surface parking runoff of 6.3 cfs to DDI # 0008 Yukon County storm drain line.

III. EXISTING STORM DRAIN SYSTEMS DESCRIPTION AND CAPACITY

Existing as-built record drawings and hydrology maps were obtained from LACDPW to characterize the storm drains that currently serve the project sites. The as-built plans can be found in [Appendix E](#).

The existing storm drain systems serving the project sites are maintained by the LACDPW and include Project 4401 Line A, Project 4402 & 681 and Project DDI # 8. These are located adjacent to the project site along Prairie and Doty Avenues. Refer to *Figure 2 — Existing Storm Drains* for existing off-site storm drain infrastructure.

The table below summarizes the storm drain infrastructure adjacent to the project site based on the latest available as-built and record drawings reviewed to date.

Location	Size	Notes
Prairie Avenue	LACDPW Project 4402 & 681- 60” diameter storm drain that flows south along Prairie Avenue.	Collects runoff flowing to Prairie from offsite properties as well as majority of proposed site
Doty Avenue	LACDPW Project 4401, Line A – 7’-0” W x 9’-0” H RCB that flows south along Doty Ave	Accepts flows from Projects 4401, Line B and MTD 922 north of Century Avenue and runoff to Doty Avenue from adjacent properties
East of Doty Avenue	LACDPW Project DDI # 8 – 4’-3” W x 4’-0” H RCB	Flowing south parallel and to the east of Doty Avenue

An information request was submitted to the LACDPW to obtain information regarding capacities of the existing storm drain systems listed above and the allowable Q’s that can be discharged from the project sites to these storm drain systems. Refer to [Appendix A](#) for a summary of the allowable storm water discharges to Project 4401, Project 4402 and DDI #8 prepared and approved on October 2017 by the LACDPW Design Division.

IV. PROJECT DESCRIPTION

Project Condor is a mixed-use project that includes a multi-purpose sport arena with auxiliary structures including retail, office buildings, restaurants, parking structures and plaza areas. The project consists of 71,000 sq. ft. of office space, 25,000 sq. ft. of retail space, 15,000 sq. ft. of food services, 85,000 sq. ft. of practice facilities, 15,000 sq. ft. of community space, 25,000 sq. ft. of sports medicine clinic and an 18,000-seat arena, a parking structure and substantial amounts of surface parking.

V. HYDROLOGY ANALYSIS AND CALCULATIONS

Hydrologic calculations were performed utilizing the LACDPW Hydrology Manual dated January 2006 and county HydroCalc Program Method. Per the LACDPW hydrology Manual, The Capital Flood Level of Protection produced by a 50-year frequency design storm is applied to proposed on-site drainage areas and facilities in sump conditions.

The existing and post-development flow rates generated by the Condor project site were calculated using the LACDPW Inglewood 50-year, 24-hour isohyet (5.15 inches rainfall depth) and associated runoff coefficient curve (Soil Type No.16 for main site and No.13 for surface parking site) and are shown in [Appendix C](#) and [Appendix D](#).

VI. POST DEVELOPMENT RUNOFF AND BASIN ROUTING

Project Condor consists of three major subareas as shown in *Figure 4 — Post-Development Hydrology Map*, with each of the project sites consisting of separate subareas. The main project site consists of two major drainage area, denoted as DA “A-3” and DA “A-4”, with a contributing runoff to the existing Project 681 storm drain lines. The parking structure site consists of another two major drainage area denoted as DA “A-5” and DA “A-6” with a contributing runoff to the existing Project 4402 & 681 storm drain lines. The parking lot site consists of another two major drainage area denoted as DA “A-1” and DA “A-2”, with a contributing runoff to the existing DDI #8 storm drain system. Design peak flows were calculated for a 50-year frequency design storm for basin routing, refer to [Appendix D](#) f or supporting calculations.

As shown in [Appendix C](#) and [Appendix D](#), the post-development runoff quantities exceed the pre-development runoff quantities. Additionally, the post-development peak flows for proposed drainage sub-areas exceed the approved allowable discharge rates to existing LACDPW Project 4402 & 681 and DDI #8 systems. Therefore, on-site detention is required for all three project’s sites. For the arena site, an underground precast detention facility with an approximately 90,000 cu.ft. volume. A similar detention facility will be required for the surface parking site with

an approximately 8,500 cu. ft volume and for northerly portion of the parking structure with approximately volume of 5,300 cu. ft. volume. The southerly portion of the parking structure drainage is anticipated to be diverted to the Prairie Avenue project # 681 storm drain instead of the Freeman storm drain system. A similar detention facility will not be feasible for the southerly parking structure site, therefore the additional detention from the arena site will compensate for the southerly portion of the parking structure. The arena site detention facility outfall will be restricted to a maximum 7.9 cfs and with the parking structure peak runoff of 8.7 cfs, the total discharge to project # 681 of 16.36 cfs will not exceed the allowable flow rate of 16.96 cfs. The surface parking lot and northerly portion of the parking structure detention facilities will have maximum outfall rates of 4.8 cfs and 2.2 cfs, respectfully.

Summary of allowable flow rates and project contributing flow rates table from all detention facilities can be found in Appendix D.

VII. SUMMARY AND CONCLUSION

The results of this study are summarized in following table.

Table 1- Allowable Runoff, existing and proposed flows

ITEM	Project 4402 (cfs)	Project 681 (cfs)	DDI#8 (cfs)	TOTAL (CFS)
Proposed Onsite 50-year Flow into Pipeline-prior to detention (Figure 4)	6.6 (Sub-area A6)	41.0 (Sub-areas A3, A4 & A5)	10.7 (Sub-areas A1 & A2)	58.3
Proposed Onsite 50-year Flow into Pipeline (HydroCalc, Figure 5) *	2.2 (Sub-area A6 detention outfall)	16.6 (Sub-areas A3 & A4 detention outfall & A5)	4.8 (Sub-areas A1 & A2 detention outfall)	23.6
LACDPW Allowable Flow Rates (Appendix B)	2.4	16.9	5.1	24.4

*Proposed runoff shown in the table are the maximum outfalls from detention facilities or proposed sites.

The Los Angeles County Department of Public Works, Design Division provided a confirmation of the allowable flow rates that may be discharged to each of the downstream storm drains adjacent to the project site, refer to Appendix A for the summary noticed provided by the LACDPW.

Based on our preliminary post-developed hydrology calculations the following has been observed:

- ∞ The design outfall from the arena site detention facility and southerly portion of the parking structure for Project 681 is approximately 16.6 cfs and the LACDPW allowable limit is 16.9 cfs.
- ∞ The design outfall from the northerly portion of the parking structure facility for Project 4402, Line C is approximately 2.2 cfs and the LACDPW allowable limit is 2.4 cfs.
- ∞ The design outfall from the surface parking site detention facility for DDI #8 is approximately 4.8 cfs and the LACDPW allowable limit is 5.1 cfs.

In summary, Project Condor will discharge 2.2 cfs and 16.6 cfs to Project 4402 & 681 and 4.8 cfs to DDI # 8, all discharge rates being within the allowable limits of 2.4cfs, 16.9 cfs and 5.1 cfs, respectively, as set by the LACDPW. Therefore, the existing downstream storm drain lines have adequate capacity for the proposed discharge from the project site. On-site storm drain pipelines and detention facilities should be designed to provide the capital flood level of protection from runoff from a 50-year frequency design storm.

The information contained within this report will be used for obtaining future connection permits to Los Angeles County Flood Control District (LACFCD) facilities and represents the existing hydrology and hydraulic data that will be used for future on-site storm drainage design.



Figures

Figure 1 — Vicinity Map

Figure 2 — Existing Storm Drains

Figure 3 — Pre-Development Hydrology Map

Figure 4 — Post-Development Hydrology Map

Figure 5 — Proposed Onsite Runoff Distribution to Existing Storm Drain Lines

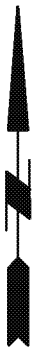
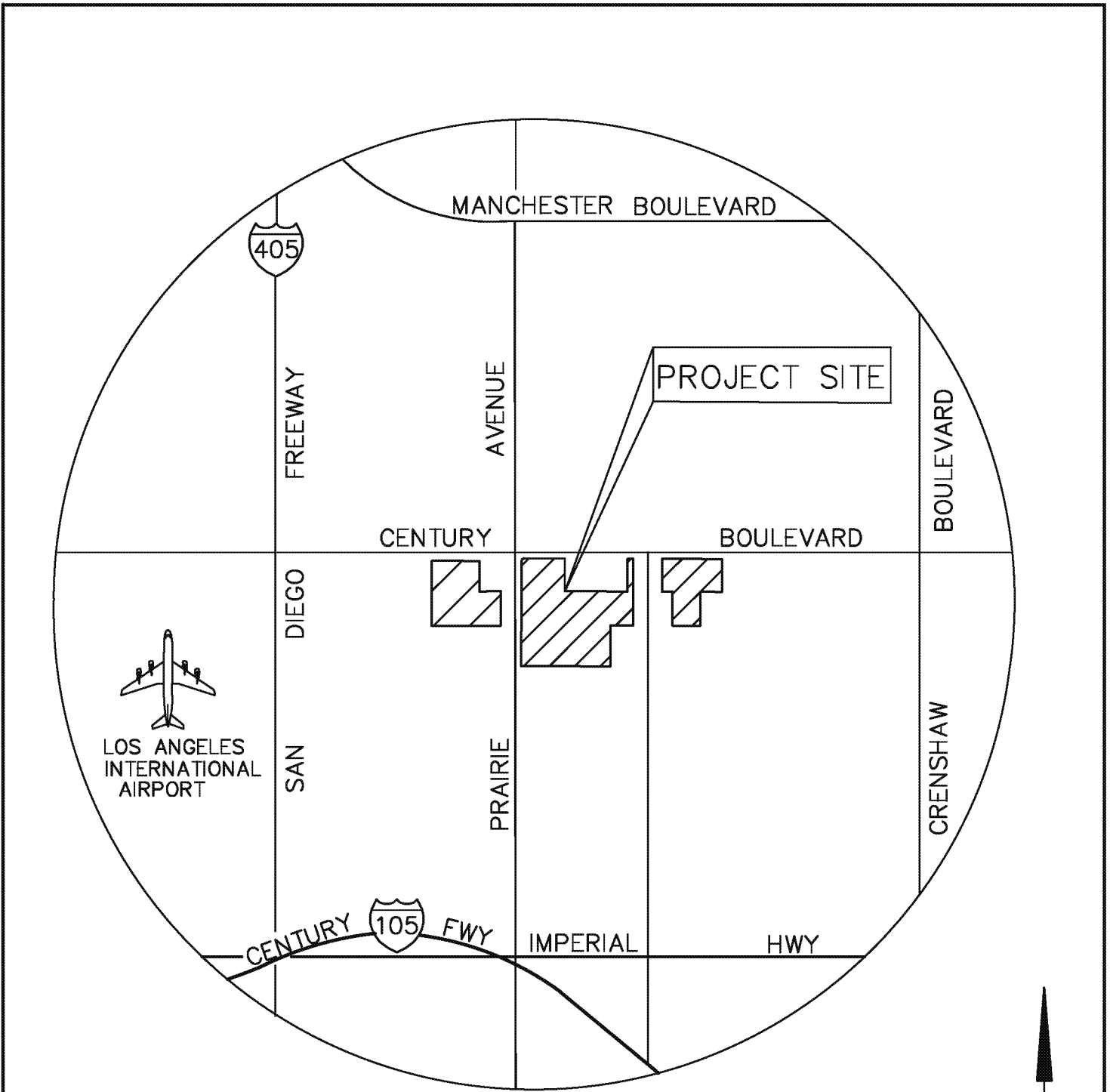
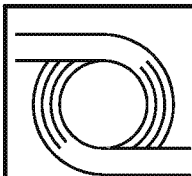


FIGURE 1



D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 106
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

PROJECT CONDOR

VICINITY MAP

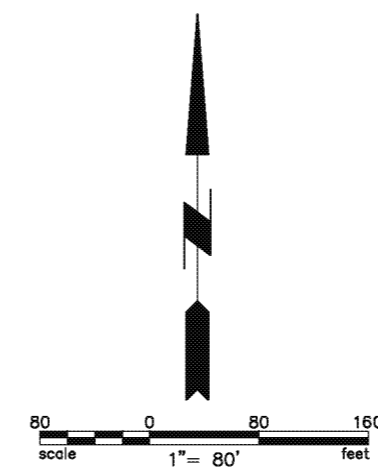
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DATE:	08/22/18
SHT NO.:	1 OF 1



SUBAREA	ACREAGE	SOIL TYPE	LENGTH	SLOPE	Q25
A1	10.7	16	965'	0.004	11.8 cfs
A2	6.2	16	910'	0.004	6.6 cfs
A3	5.1	13	685'	0.004	6.3 cfs
A4	3.3	16	427'	0.005	4.8 cfs
A5	2.2	16	600'	0.003	2.6 cfs
TOTAL	27.5	---	---	---	32.1 cfs

LEGEND

- ▬▬▬▬▬ PROJECT STUDY LIMITS
- ▬▬▬▬▬ EXISTING STORM DRAIN LINES
- ⊘ AREA
ACRES
- ▭ DRAINAGE SUBAREA



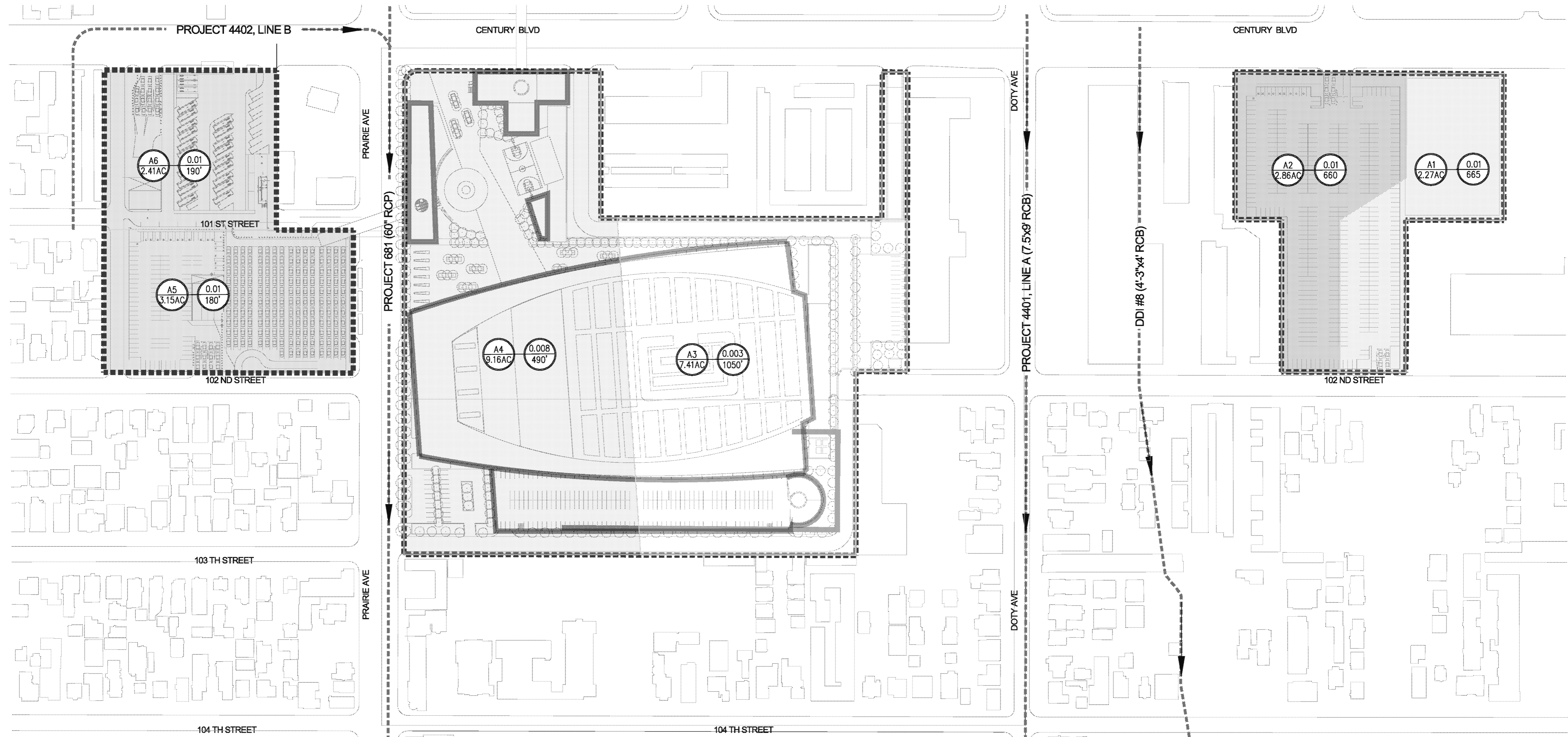
D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 108
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

PROJECT CONDOR
**PRE-DEVELOPMENT
 HYDROLOGY MAP**

FIGURE 3






SCALE: 1" = 80'
 DATE: 08/21/18
 SHEET NO.: 1 OF 1

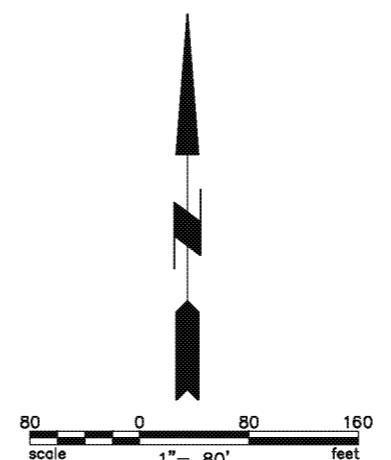
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 User: jboyle
 Plot Date: 08/22/2018 11:01:10 AM



SUBAREA	ACREAGE	LENGTH	SLOPE	Q ₁₀
A1	2.3	665'	0.010	4.7 cfs
A2	2.9	660'	0.010	6.0 cfs
A3	7.4	1050'	0.004	12.1 cfs
A4	9.2	490'	0.008	20.2 cfs
A5	3.2	190'	0.010	8.7 cfs
A6	2.4	180'	0.010	6.6 cfs
TOTAL	27.4	---	---	58.3 cfs

LEGEND

-  PROJECT STUDY LIMITS
-  EXISTING STORM DRAIN LINES
-  DRAINAGE AREA
-  AREA ACRES
-  SLOPE LENGTH



D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 108
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

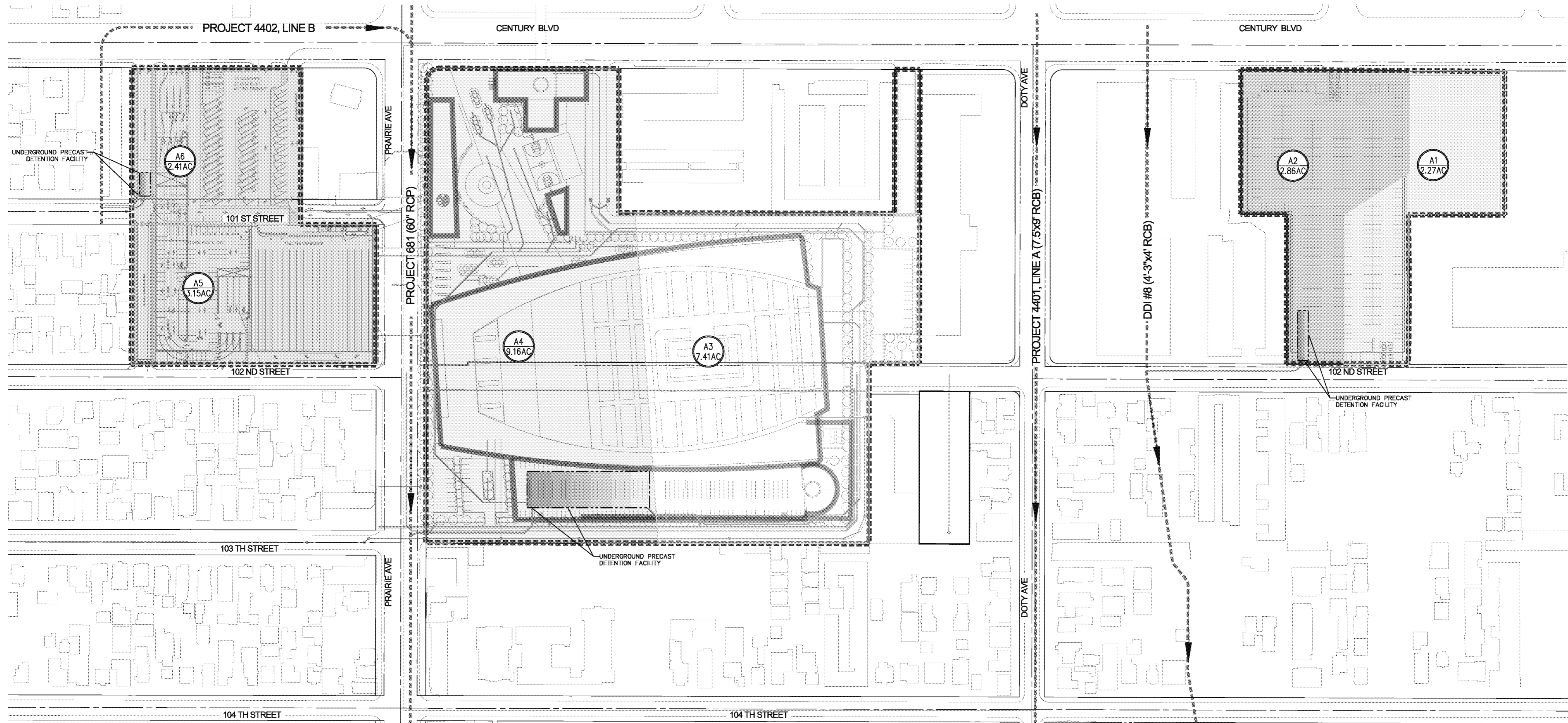
PROJECT CONDOR

**POST-DEVELOPMENT
 HYDROLOGY MAP**

SCALE: 1" = 80'
 DATE: 08/21/18
 SHEET NO.: 1 OF 1

FIGURE 4

Drawn: Noreen, SA (12/20/18) \CA\17301\Condor\Map\1808\PostDev\hydrology map_08-21-18.dwg
 Plot: 08/22/2018 10:46:56 AM by: Bora



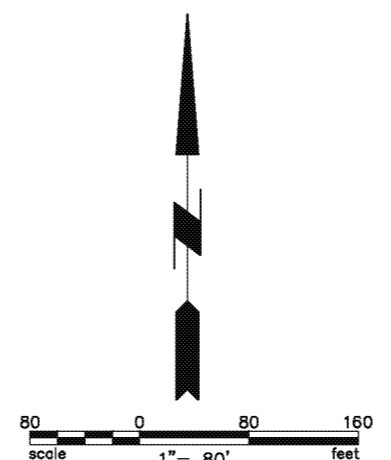
SUBAREA	ACREAGE	LENGTH	SLOPE	Q _{av}
A1	2.3	665'	0.010	4.7 cfs
A2	2.9	660'	0.010	6.0 cfs
A3	7.4	1050'	0.004	12.1 cfs
A4	9.2	490'	0.008	20.2 cfs
A5	3.2	190'	0.010	8.7 cfs
A6	2.4	180'	0.010	6.6 cfs
TOTAL	27.4			58.3 cfs

EXISTING STORM DRAIN LINE	CONTRIBUTING ON-SITE DRAINAGE AREA	ALLOWABLE DISCHARGE TO EXISTING LINE (cfs)	CONTRIBUTING FLOW (cfs)	NOTE
PROJECT DDI #0008	A1 & A2	5.10	4.8	SUFFICIENT SD CAPACITY
PROJECT 4402 LINE C	A6	2.40	2.2	SUFFICIENT SD CAPACITY
PROJECT 681	A3, A4 & A5	16.90	16.6*	SUFFICIENT SD CAPACITY

* TOTAL RUNOFF FROM DRAINAGE AREAS A3 & A4 DETENTION FACILITY OUTFALL OF 7.9 CFS AND DRAINAGE AREA A6 RUNOFF OF 8.7 CFS.

LEGEND

- PROJECT STUDY LIMITS
- EXISTING STORM DRAIN LINES
- DRAINAGE SUB-AREA
- AREA (ACRES)
- SLOPE (LENGTH)



D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 106
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

PROJECT CONDOR

PROPOSED ONSITE RUNOFF DISTRIBUTION TO EXISTING STORM DRAIN LINES

SCALE: 1" = 80'
 DATE: 08/21/18
 SHEET NO.: 1 OF 1

FIGURE 5

Drawn: [Name], Checked: [Name], Date: 08/21/18, Scale: 1" = 80', Plot Date: Aug 23, 2018, 4:35pm by [Name]



Appendix A

Off-Site Hydrology and LACDOW Information Request Summary

Figure 2 — Existing Storm Drains




Offsite Tributary Area Map for Project 4402 & 681

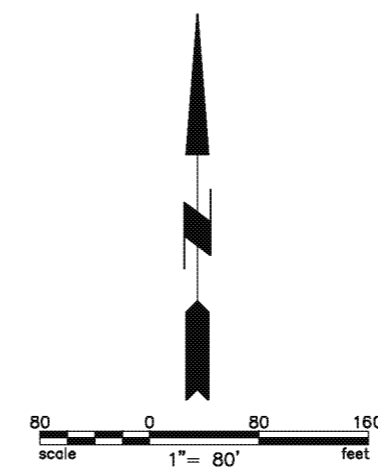
Offsite Tributary Area Map for Project DDI #8

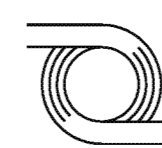
Information Request Summary



LEGEND

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-  EXISTING STORM DRAIN LINES
-  EXISTING CITY OF INGLEWOOD STORM DRAIN LINE

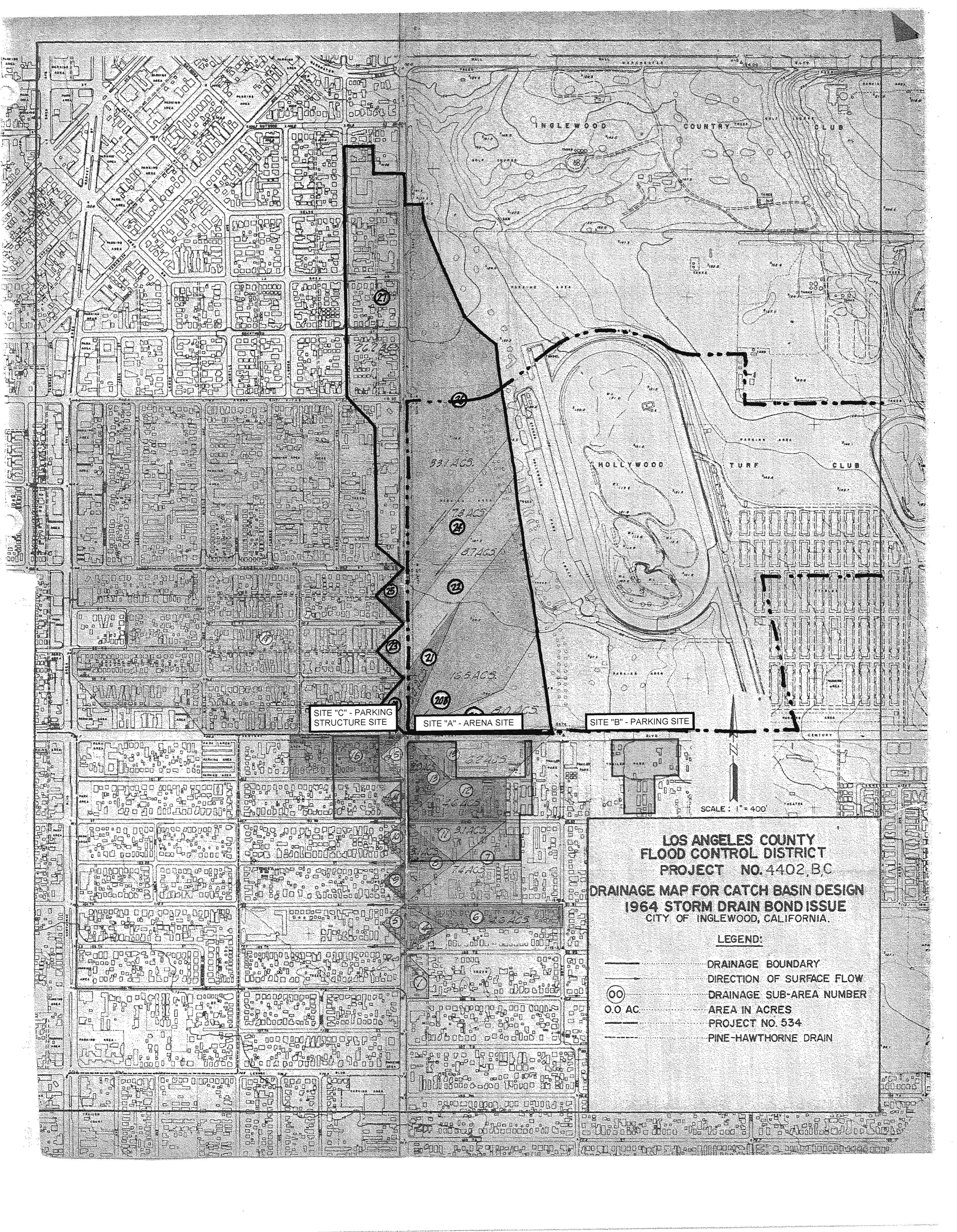



D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 106
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

PROJECT CONDOR	
EXISTING STORM DRAINS	
SCALE: 1" = 80'	DATE: 08/22/18
SHEET NO.:	1 OF 1

FIGURE 2

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 Date: 08/22/18 10:48am



SITE "C" - PARKING
STRUCTURE SITE

SITE "A" - ARENA SITE

SITE "B" - PARKING SITE

SCALE : 1" = 400'

**LOS ANGELES COUNTY
FLOOD CONTROL DISTRICT
PROJECT NO. 4402, B/C
DRAINAGE MAP FOR CATCH BASIN DESIGN
1964 STORM DRAIN BOND ISSUE
CITY OF INGLEWOOD, CALIFORNIA.**

LEGEND:

- DRAINAGE BOUNDARY
- DIRECTION OF SURFACE FLOW
- OO DRAINAGE SUB-AREA NUMBER
- 0.0 AC. AREA IN ACRES
- PROJECT NO. 534
- PINE-HAWTHORNE DRAIN

INGLEWOOD COUNTRY CLUB

BOULDER EXBT
72" S.D. HERE
C.P.

24a

23

24A

25

26

25A

28

HOLLYWOOD TURF

27A

27

32

29

29A

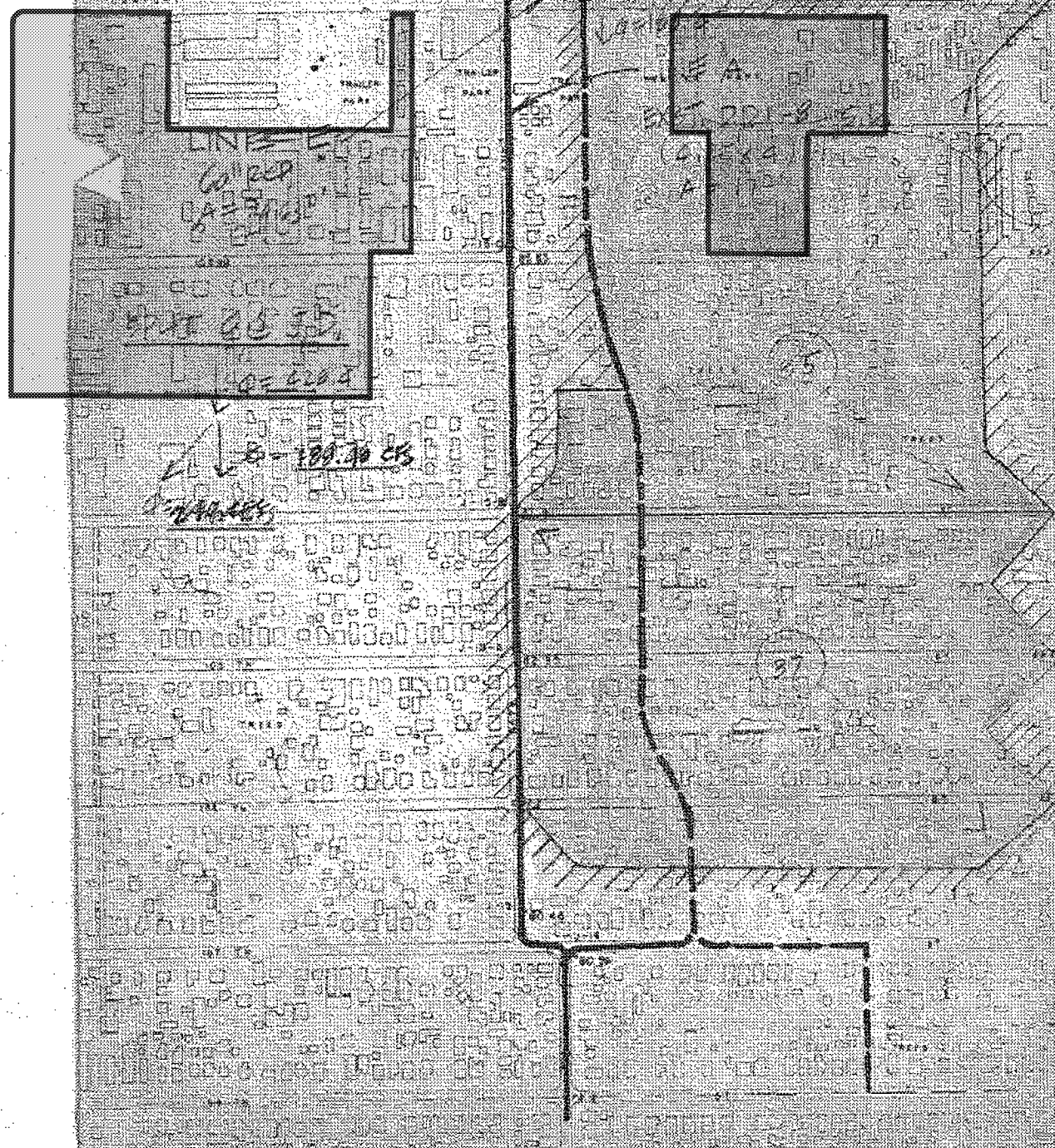
EXIST. 54" S.D.

EXIST. 60" S.D.
C.P.

30

SITE "A" - ARENA SITE

SITE "B" - PARKING SITE



HYDROLOGY MAP
PROJECT 4401 UNIT 1
INTERIM DRAIN LINE "A" (ASSUMING
LINE B WILL NOT BE BUILT)
SCALE 1" = 400'
(PRELIM)
(EXTENDED TO ...
PICK UP INTERIM
FLOW AT DDIES)



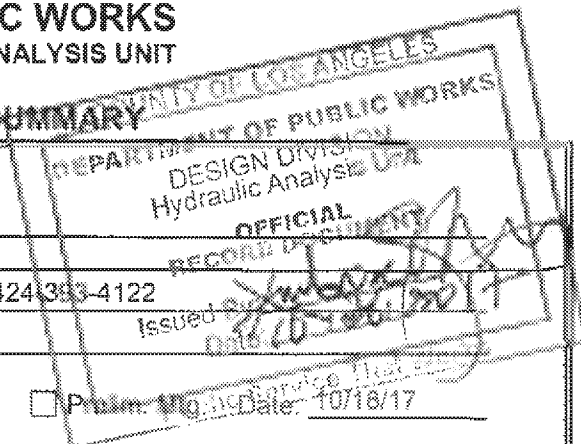
**LOS ANGELES COUNTY
DEPARTMENT OF PUBLIC WORKS
DESIGN DIVISION – HYDRAULIC ANALYSIS UNIT**

Office Use Only	
<input type="checkbox"/> Sent	Initials: _____
<input type="checkbox"/> Fax	<input checked="" type="checkbox"/> Email <input type="checkbox"/> Other: _____
Date: _____	Time: _____

INFORMATION REQUEST SUMMARY

INFORMATION REQUESTED BY

*Requester's Name: Boris Tantchev
 Company: D & D Engineering, Inc.
 *Phone Number: 8182-653-8666 Fax Number: 424-383-4122
 *Email: BTantchev@DandDEngineeringInc.com



Method of Contact: Walk-in Phone Fax Email Print MLG, JCC Date: 10/18/17

Intended Use: Research and Due Diligence

Proposed Project Type: Land Development - Mixed Use and Retail/Com Acreage Involved: 26

*Will information be used in any litigation? YES NO
 Case Info. Name: _____ No: _____ Location: _____

INFORMATION REQUESTED (Attach Assessor Map)

LACFCD Facility: Name: LACFCD Proj #681 and Proj #4401-U1 Line A, and DDI 0008
 Unit: _____ Line: _____ Station: _____
 City: Inglewood
 *Street/Cross-street: Century Blvd and Prairie Avenue
 *Thomas Guide: Page: 703 Grid: E5 Site Map/Plans Submitted
 Info. Requested: Allowable Q

*Required Information. See Page 2 of 2 for Instructions.

BELOW SECTION TO BE COMPLETED BY THE HYDRAULIC ANALYSIS UNIT

INFORMATION PROVIDED: *Allowable q per acre.*

REFERENCES SEARCHED: *Projects 534, 4401, 4402 and DDI 8 Hydrology.*

COMMENTS, ETC:

*Allowable q per acre for connection to Project 4402 = 1.00 cfs.
 Allowable q per acre for connection to Project 4401 = 0.70 cfs
 Allowable q per acre for connection to DDI 8 = 1.00 cfs.*

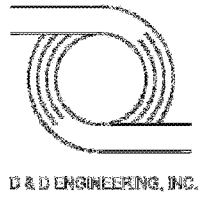
INFORMATION PROVIDED BY: *Ambrose C Ajaelo PE*

Date: *10/26/2017*

INFORMATION REVIEWED BY:

Date:





Appendix B

*LACDPW, 2006 Hydrology Manual, Appendix B
Hydrologic Maps, 08 Inglewood*

34° 00' 00"

HOLLYWOOD 1-H1.18

-118° 22' 30"

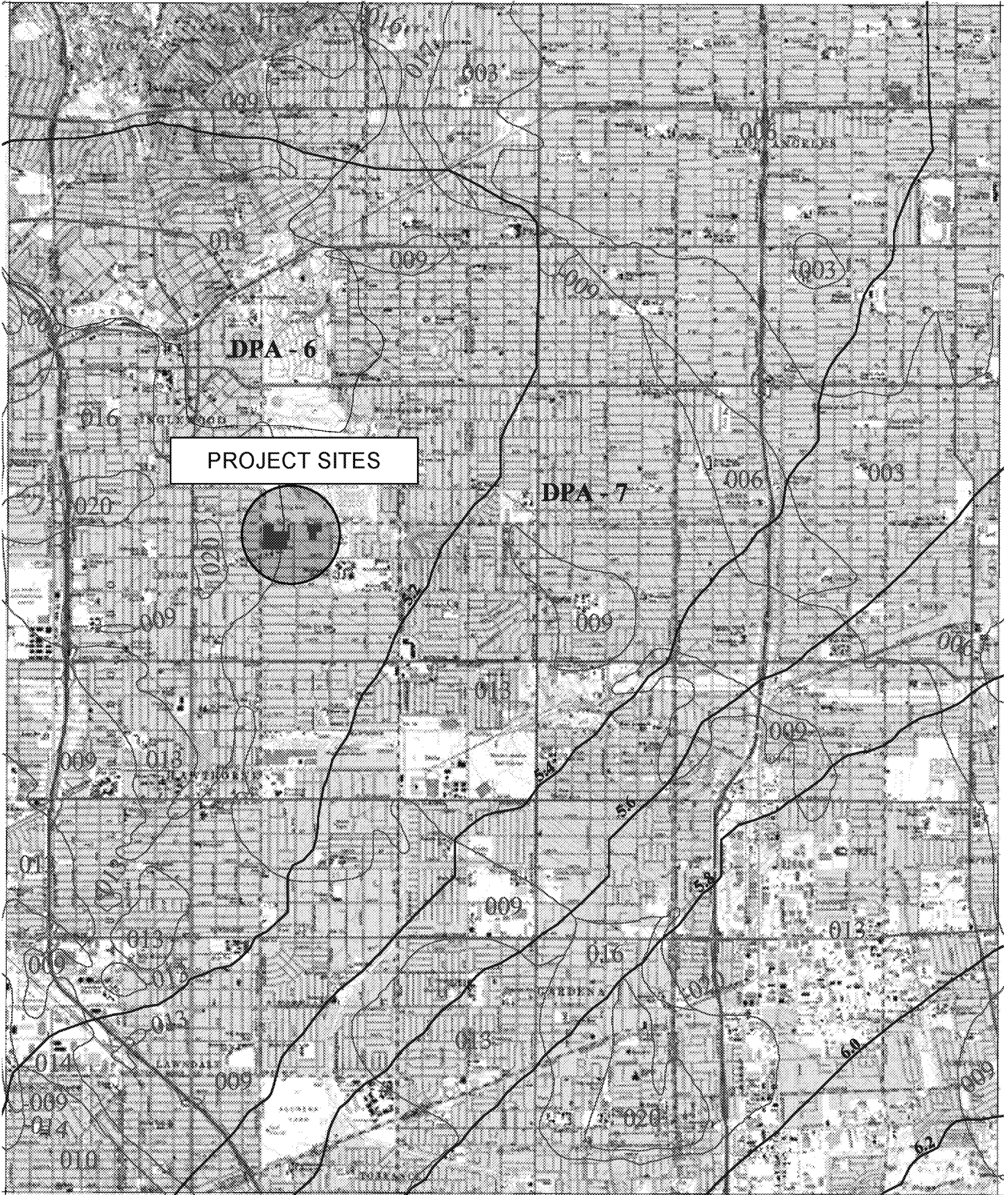
VENICE 1-H1.7

SOUTH GATE 1-H1.9

-118° 15' 00"

TORRANCE 1-H1.4

33° 52' 30"



016

SOIL CLASSIFICATION AREA

7.2

INCHES OF RAINFALL

DPA - 6

DEBRIS POTENTIAL AREA

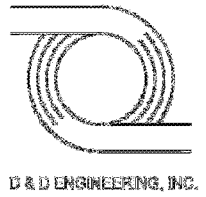
1 0 1 2 Miles

25-YEAR 24-HOUR ISOHYET REDUCTION FACTOR: 0.878
10-YEAR 24-HOUR ISOHYET REDUCTION FACTOR: 0.714

INGLEWOOD 50-YEAR 24-HOUR ISOHYET

1-H1.8





Appendix C

Pre-Development Runoff Calculations

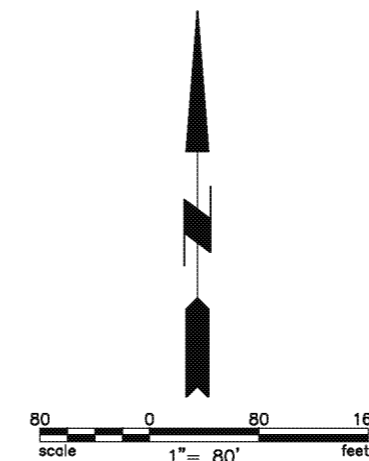
*Figure 3 — Pre-Development Hydrology Map
Sub-areas HydroCalc Worksheets*



SUBAREA	ACREAGE	SOIL TYPE	LENGTH	SLOPE	Q25
A1	10.7	16	965'	0.004	11.8 cfs
A2	6.2	16	910'	0.004	6.6 cfs
A3	5.1	13	685'	0.004	6.3 cfs
A4	3.3	16	427'	0.005	4.8 cfs
A5	2.2	16	600'	0.003	2.6 cfs
TOTAL	27.5	---	---	---	32.1 cfs

LEGEND

- PROJECT STUDY LIMITS
- EXISTING STORM DRAIN LINES
- AREA ACRES
- DRAINAGE SUBAREA



<p>D & D ENGINEERING, INC. 8901 S. LA CIENEGA BLVD, SUITE 106 INGLEWOOD, CA 90301 Phone: 424-351-6800</p>	<p>PROJECT CONDOR</p> <p>PRE-DEVELOPMENT HYDROLOGY MAP</p>	<p>SCALE: 1" = 80'</p> <p>DATE: 08/21/18</p> <p>SHEET NO.: 1 OF 1</p>
	<p>FIGURE 3</p>	

Drawn: Nanda, M. V. 1/20/18; Checked: V. 1/20/18; Calculated: M. V. 1/20/18; Report: Pre-dev hydrology map - 08-21-18.dwg
Last Update: Aug 22, 2018 - 7:01pm by Boris

Peak Flow Hydrologic Analysis

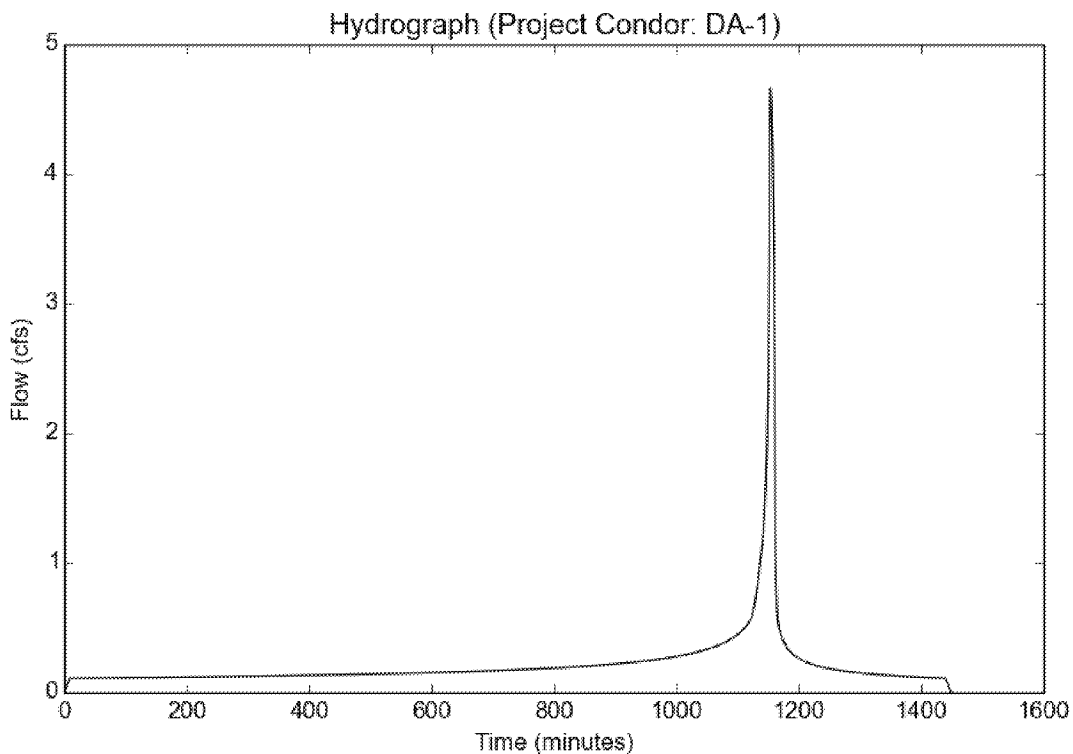
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Report Proposed Conditions.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	DA-1
Area (ac)	2.27
Flow Path Length (ft)	665.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.4
Soil Type	13
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	5.15
Peak Intensity (in/hr)	2.3309
Undeveloped Runoff Coefficient (Cu)	0.8687
Developed Runoff Coefficient (Cd)	0.8812
Time of Concentration (min)	9.0
Clear Peak Flow Rate (cfs)	4.6628
Burned Peak Flow Rate (cfs)	4.6628
24-Hr Clear Runoff Volume (ac-ft)	0.4473
24-Hr Clear Runoff Volume (cu-ft)	19482.7616



Peak Flow Hydrologic Analysis

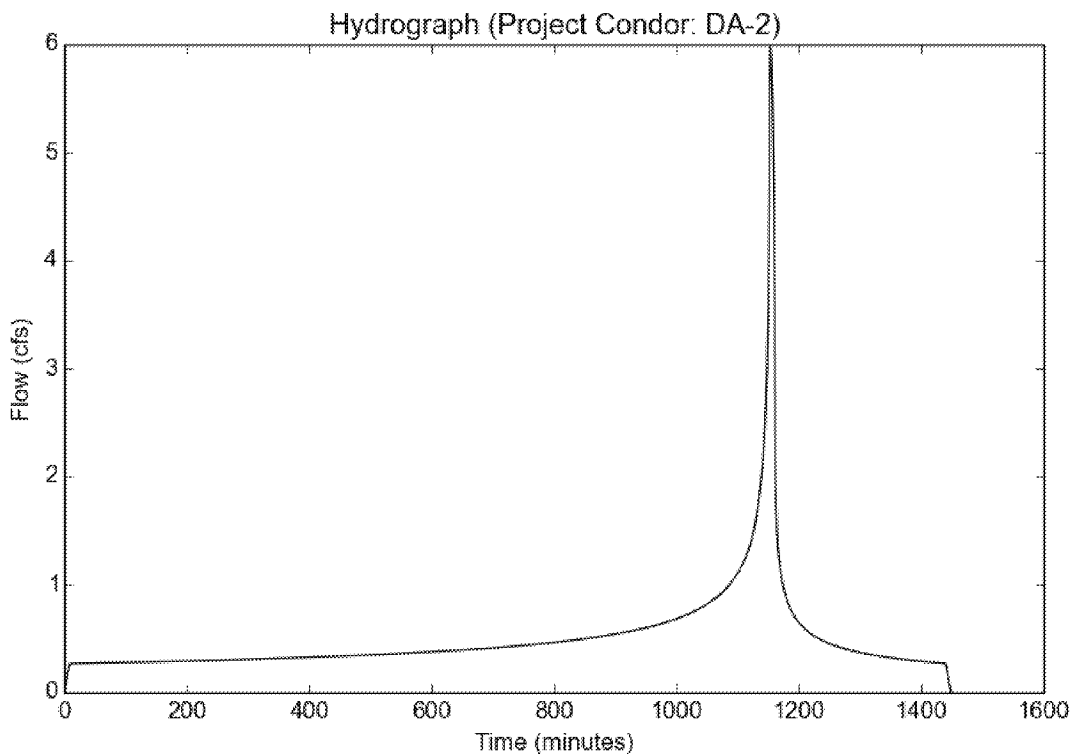
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Report Proposed Conditions.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	DA-2
Area (ac)	2.86
Flow Path Length (ft)	660.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.9
Soil Type	13
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	5.15
Peak Intensity (in/hr)	2.3309
Undeveloped Runoff Coefficient (Cu)	0.8687
Developed Runoff Coefficient (Cd)	0.8969
Time of Concentration (min)	9.0
Clear Peak Flow Rate (cfs)	5.979
Burned Peak Flow Rate (cfs)	5.979
24-Hr Clear Runoff Volume (ac-ft)	1.0069
24-Hr Clear Runoff Volume (cu-ft)	43859.4344



Peak Flow Hydrologic Analysis

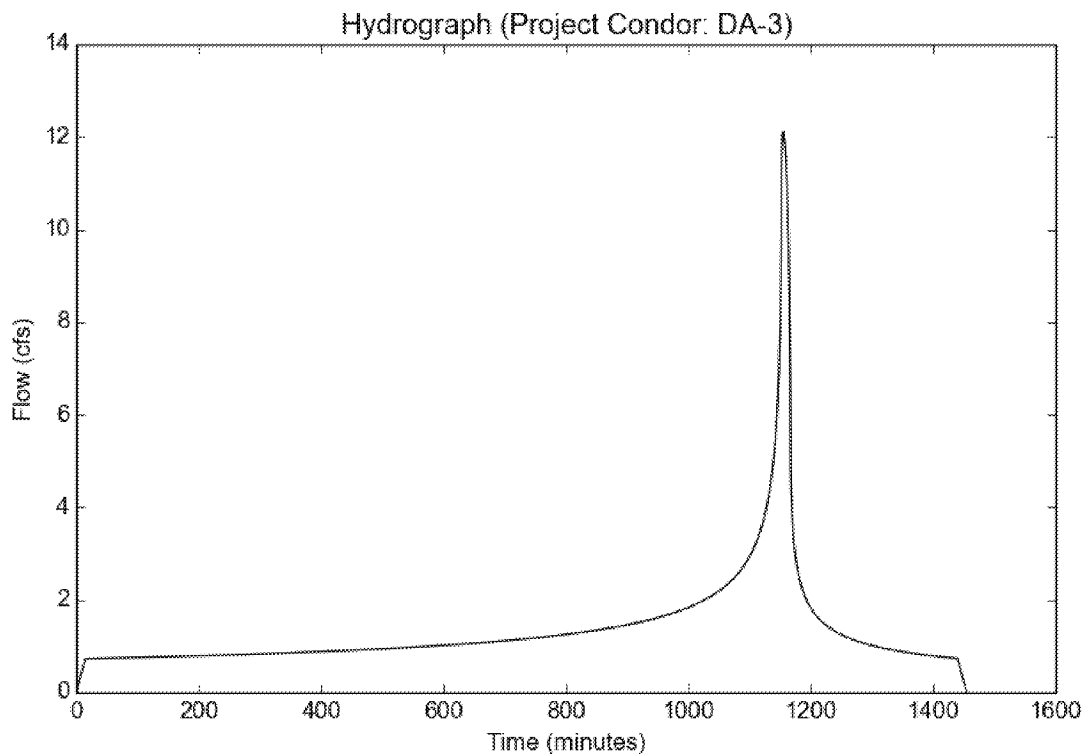
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Report Proposed Conditions.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	DA-3
Area (ac)	7.41
Flow Path Length (ft)	1050.0
Flow Path Slope (vft/hft)	0.0035
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.95
Soil Type	16
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	5.15
Peak Intensity (in/hr)	1.8334
Undeveloped Runoff Coefficient (Cu)	0.7308
Developed Runoff Coefficient (Cd)	0.8915
Time of Concentration (min)	15.0
Clear Peak Flow Rate (cfs)	12.1122
Burned Peak Flow Rate (cfs)	12.1122
24-Hr Clear Runoff Volume (ac-ft)	2.7251
24-Hr Clear Runoff Volume (cu-ft)	118707.0023



Peak Flow Hydrologic Analysis

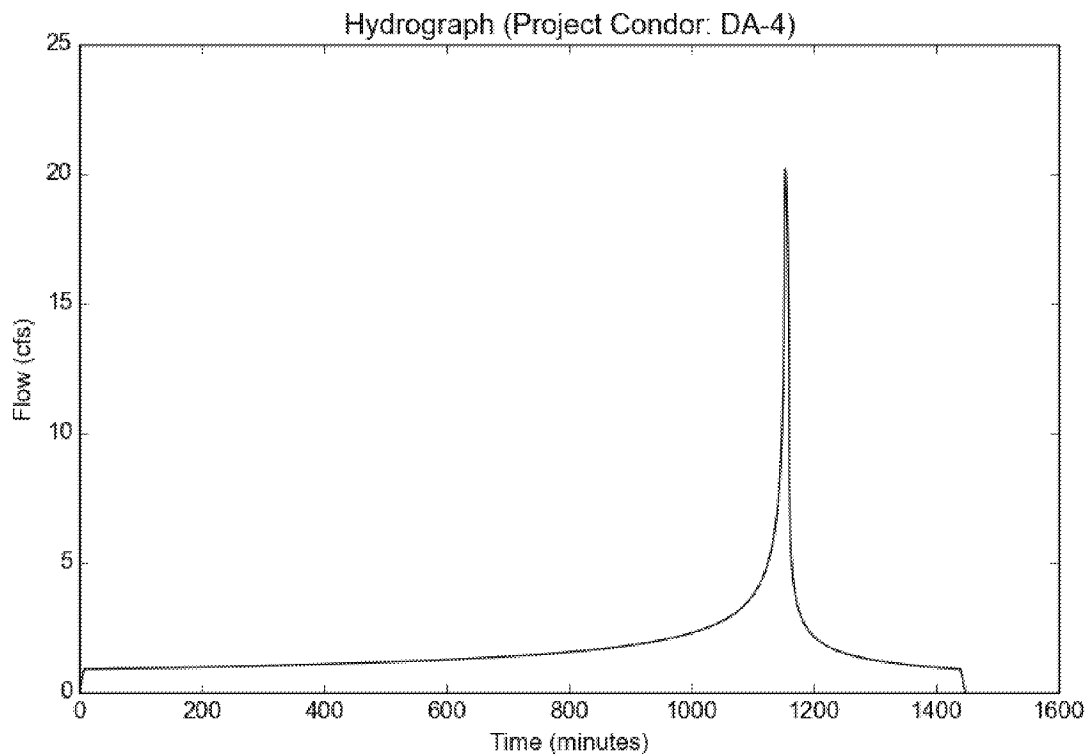
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Report Proposed Conditions.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	DA-4
Area (ac)	9.16
Flow Path Length (ft)	490.0
Flow Path Slope (vft/hft)	0.008
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.95
Soil Type	16
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	5.15
Peak Intensity (in/hr)	2.4636
Undeveloped Runoff Coefficient (Cu)	0.8069
Developed Runoff Coefficient (Cd)	0.8953
Time of Concentration (min)	8.0
Clear Peak Flow Rate (cfs)	20.205
Burned Peak Flow Rate (cfs)	20.205
24-Hr Clear Runoff Volume (ac-ft)	3.369
24-Hr Clear Runoff Volume (cu-ft)	146752.9576



Peak Flow Hydrologic Analysis

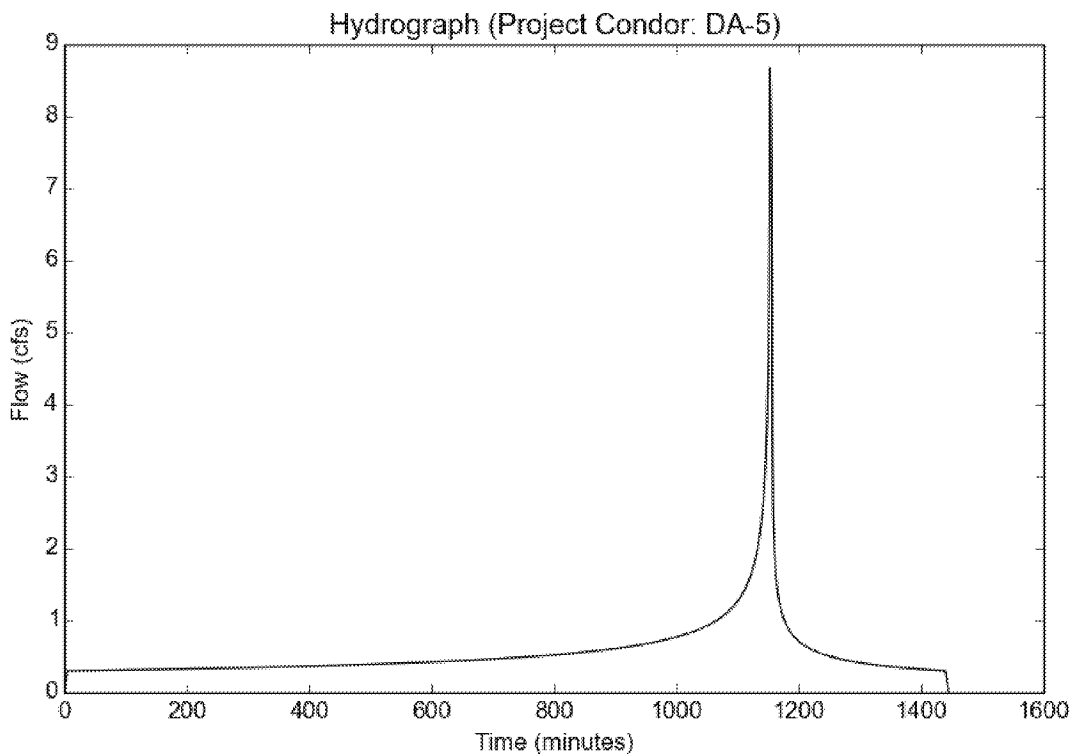
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Report Proposed Conditions.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	DA-5
Area (ac)	3.15
Flow Path Length (ft)	190.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.92
Soil Type	16
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	5.15
Peak Intensity (in/hr)	3.0726
Undeveloped Runoff Coefficient (Cu)	0.8531
Developed Runoff Coefficient (Cd)	0.8962
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	8.6746
Burned Peak Flow Rate (cfs)	8.6746
24-Hr Clear Runoff Volume (ac-ft)	1.1298
24-Hr Clear Runoff Volume (cu-ft)	49212.197



Peak Flow Hydrologic Analysis

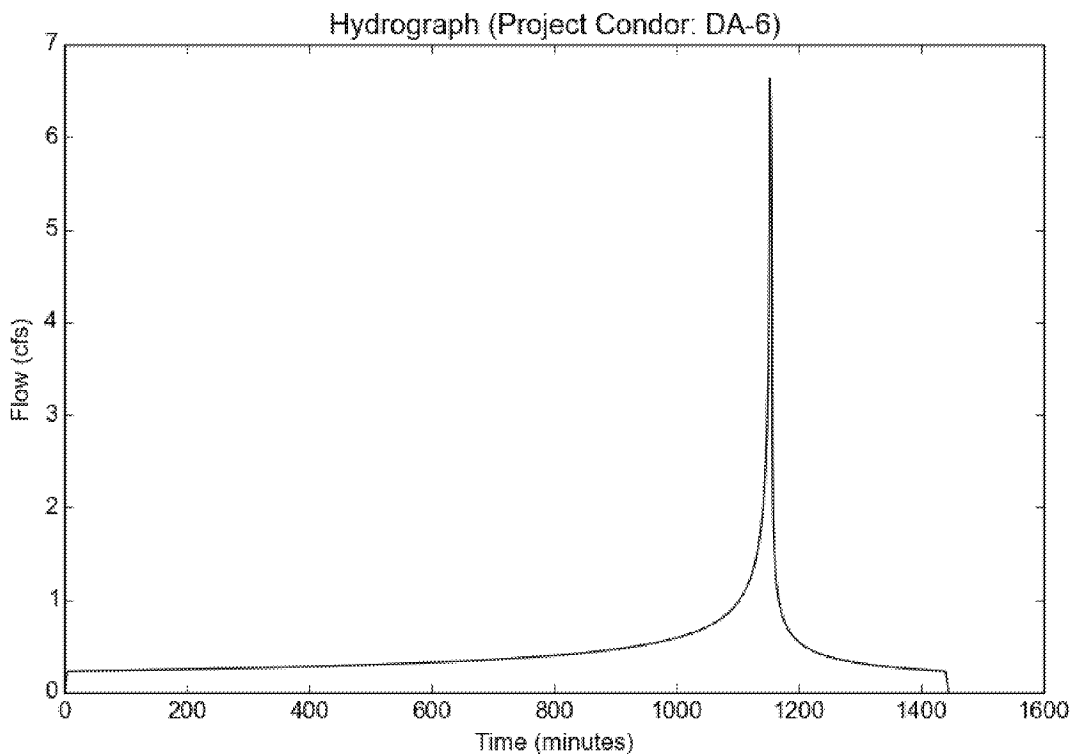
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Report Proposed Conditions.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	DA-6
Area (ac)	2.41
Flow Path Length (ft)	180.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.92
Soil Type	16
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	5.15
Peak Intensity (in/hr)	3.0726
Undeveloped Runoff Coefficient (Cu)	0.8531
Developed Runoff Coefficient (Cd)	0.8962
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	6.6368
Burned Peak Flow Rate (cfs)	6.6368
24-Hr Clear Runoff Volume (ac-ft)	0.8644
24-Hr Clear Runoff Volume (cu-ft)	37651.2365





Appendix D

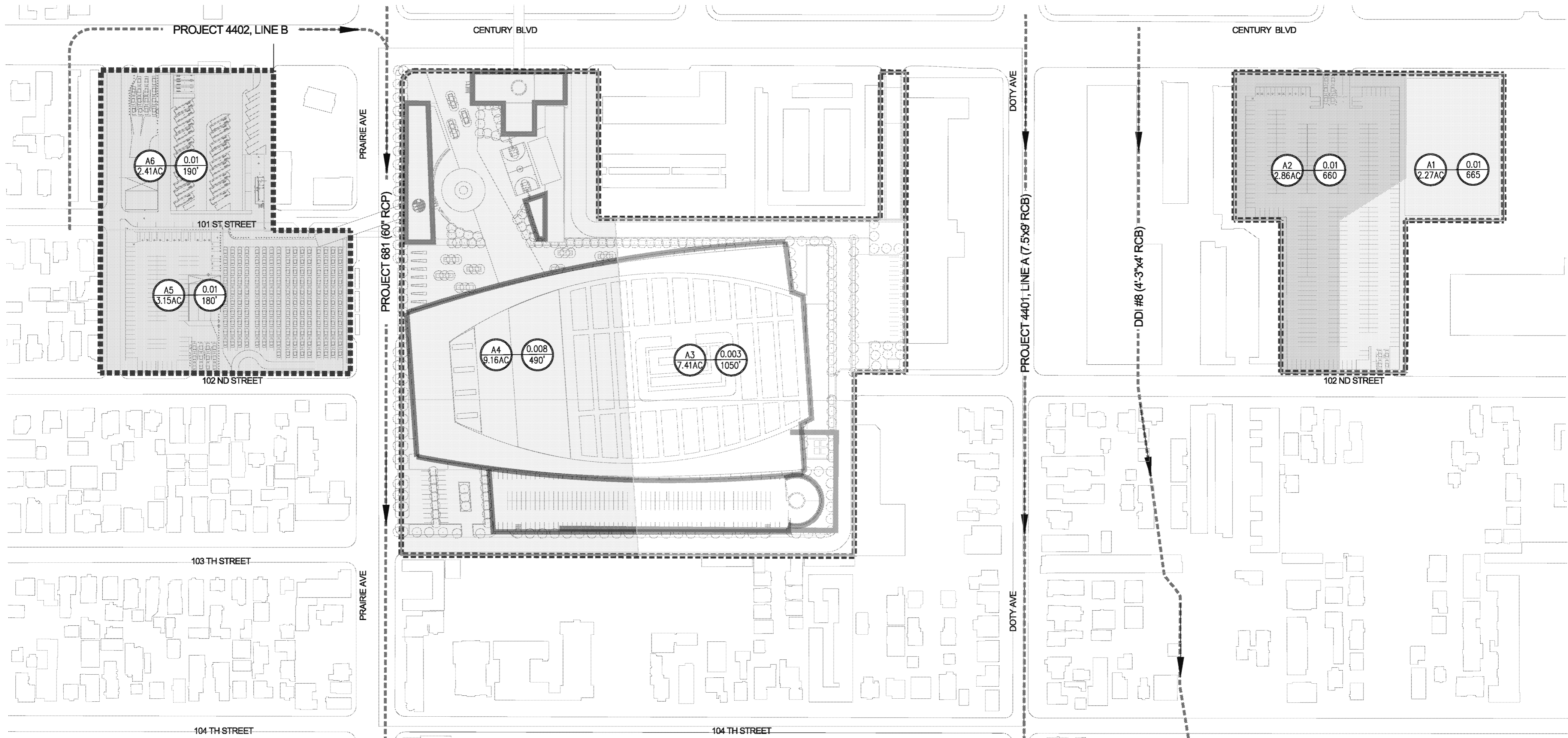
Post-Development Runoff Calculations and Basin Routing

Figure 4 — Post-Development Hydrology Map

Figure 5 — Proposed Onsite Runoff Distribution to Existing Storm Drain Lines

Sub-Areas HydroCalc Worksheets

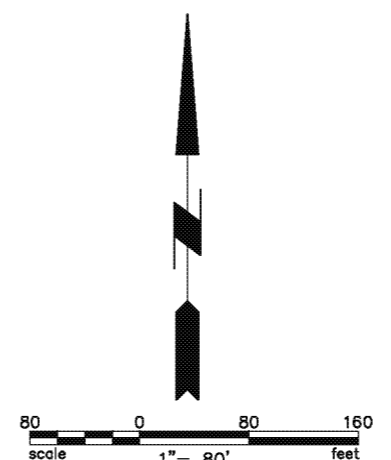
Summary of Post-Development Runoff Distribution to Existing Storm Drain Systems



SUBAREA	ACREAGE	LENGTH	SLOPE	Q ₁₀
A1	2.3	665'	0.010	4.7 cfs
A2	2.9	660'	0.010	6.0 cfs
A3	7.4	1050'	0.004	12.1 cfs
A4	9.2	490'	0.008	20.2 cfs
A5	3.2	190'	0.010	8.7 cfs
A6	2.4	180'	0.010	6.6 cfs
TOTAL	27.4	---	---	58.3 cfs

LEGEND

- PROJECT STUDY LIMITS
- EXISTING STORM DRAIN LINES
- DRAINAGE AREA
- AREA (ACRES)
- SLOPE (LENGTH)

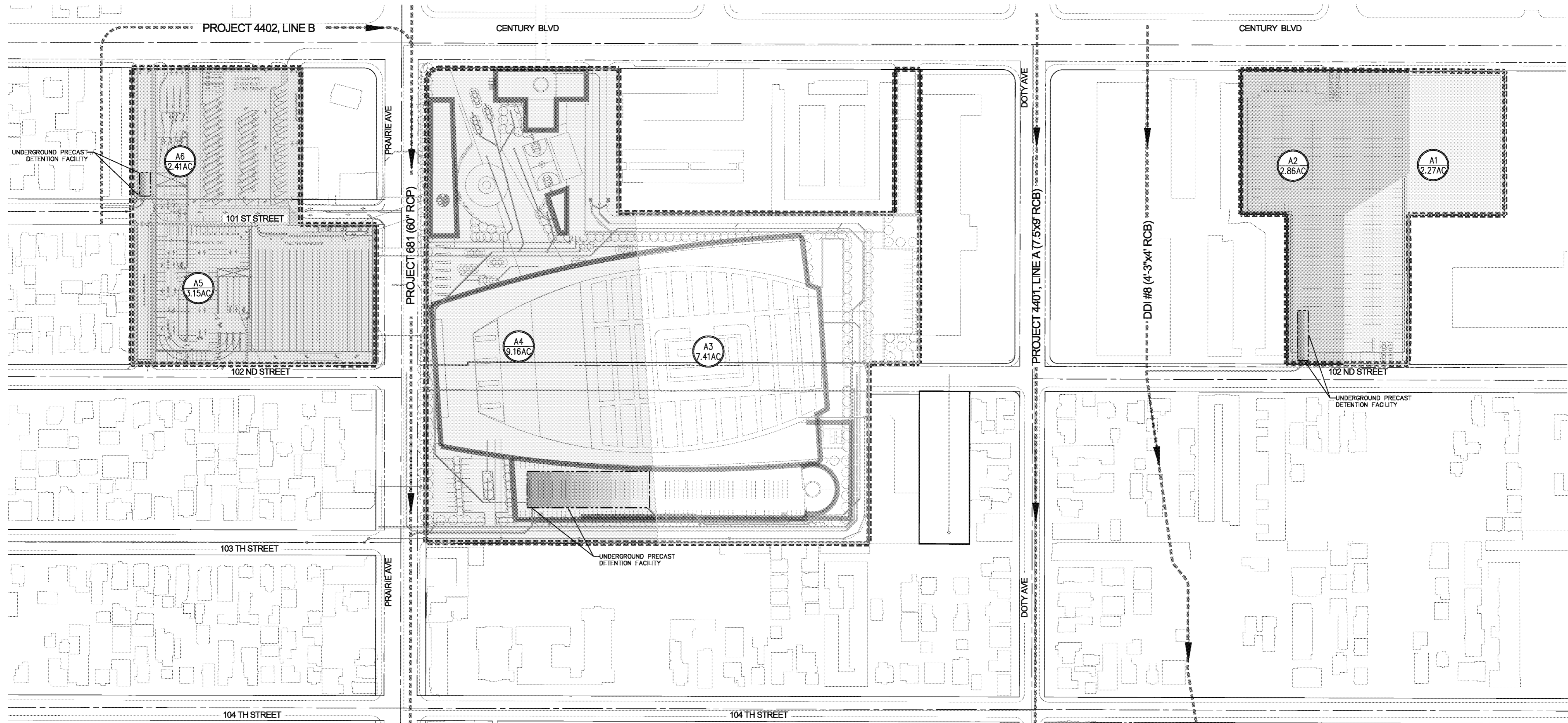


D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 108
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

PROJECT CONDOR
POST-DEVELOPMENT
HYDROLOGY MAP

SCALE: 1" = 80'
 DATE: 08/21/18
 SHEET NO.: 1 OF 1

Drawn: [Name], [Title], [Date] | Checked: [Name], [Title], [Date] | Project: [Name] | File: [Name] | Date: 08-21-18 | Scale: 1" = 80'



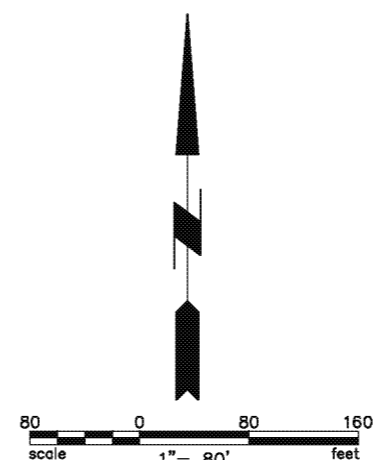
SUBAREA	ACREAGE	LENGTH	SLOPE	Q _{av}
A1	2.3	665'	0.010	4.7 cfs
A2	2.9	660'	0.010	6.0 cfs
A3	7.4	1050'	0.004	12.1 cfs
A4	9.2	490'	0.008	20.2 cfs
A5	3.2	190'	0.010	8.7 cfs
A6	2.4	180'	0.010	6.6 cfs
TOTAL	27.4			58.3 cfs

EXISTING STORM DRAIN LINE	CONTRIBUTING ON-SITE DRAINAGE AREA	ALLOWABLE DISCHARGE TO EXISTING LINE (cfs)	CONTRIBUTING FLOW (cfs)	NOTE
PROJECT DDI #0008	A1 & A2	5.10	4.8	SUFFICIENT SD CAPACITY
PROJECT 4402 LINE C	A6	2.40	2.2	SUFFICIENT SD CAPACITY
PROJECT 681	A3, A4 & A5	16.90	16.6*	SUFFICIENT SD CAPACITY

* TOTAL RUNOFF FROM DRAINAGE AREAS A3 & A4 DETENTION FACILITY OUTFALL OF 7.9 CFS AND DRAINAGE AREA A6 RUNOFF OF 8.7 CFS.

LEGEND

- PROJECT STUDY LIMITS
- EXISTING STORM DRAIN LINES
- DRAINAGE SUB-AREA
- AREA (ACRES)
- SLOPE (LENGTH)



D & D ENGINEERING, INC.
 8901 S. LA CIENEGA BLVD, SUITE 106
 INGLEWOOD, CA 90301
 Phone: 424-351-6800

PROJECT CONDOR

PROPOSED ONSITE RUNOFF DISTRIBUTION TO EXISTING STORM DRAIN LINES

SCALE: 1" = 80'
 DATE: 08/21/18
 SHEET NO.: 1 OF 1

FIGURE 5

Drawn: [Name], Checked: [Name], Date: 08/21/18
 Project: [Name], Sheet: 1 of 1

Peak Flow Hydrologic Analysis

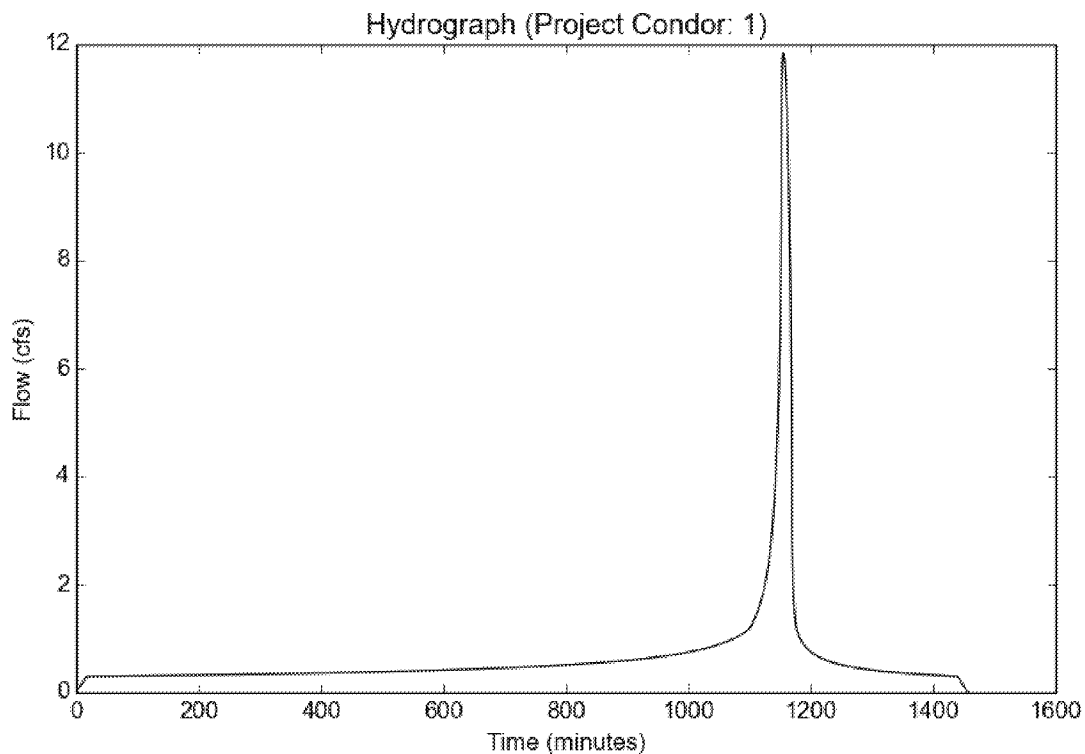
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Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	1
Area (ac)	10.7
Flow Path Length (ft)	965.0
Flow Path Slope (vft/hft)	0.004
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.22
Soil Type	16
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.5217
Peak Intensity (in/hr)	1.5178
Undeveloped Runoff Coefficient (Cu)	0.6806
Developed Runoff Coefficient (Cd)	0.7289
Time of Concentration (min)	17.0
Clear Peak Flow Rate (cfs)	11.8372
Burned Peak Flow Rate (cfs)	11.8372
24-Hr Clear Runoff Volume (ac-ft)	1.3194
24-Hr Clear Runoff Volume (cu-ft)	57473.0735



Peak Flow Hydrologic Analysis

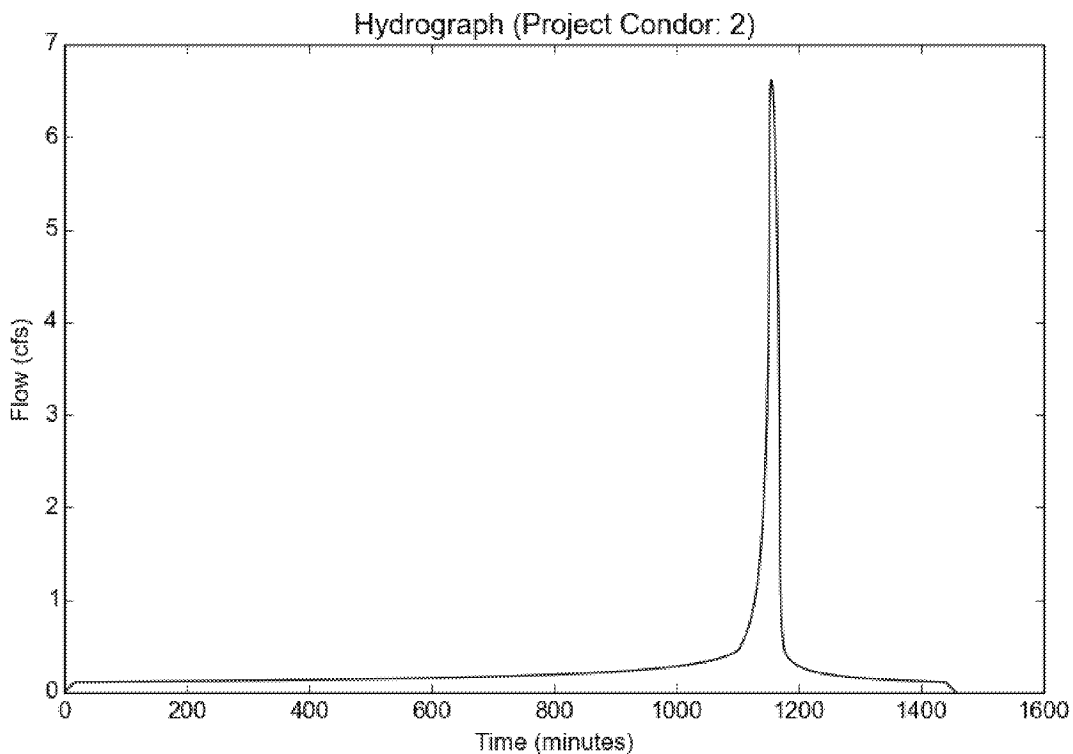
File location: M:/17001/Eng/17001/Hm/Hydrology/Conceptual Hydrocalc/EIR/Project Condor Existing Drainage Area Report.pdf
Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	2
Area (ac)	6.2
Flow Path Length (ft)	910.0
Flow Path Slope (vft/hft)	0.004
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.1
Soil Type	16
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.5217
Peak Intensity (in/hr)	1.5178
Undeveloped Runoff Coefficient (Cu)	0.6806
Developed Runoff Coefficient (Cd)	0.7026
Time of Concentration (min)	17.0
Clear Peak Flow Rate (cfs)	6.6112
Burned Peak Flow Rate (cfs)	6.6112
24-Hr Clear Runoff Volume (ac-ft)	0.5613
24-Hr Clear Runoff Volume (cu-ft)	24451.3721



Peak Flow Hydrologic Analysis

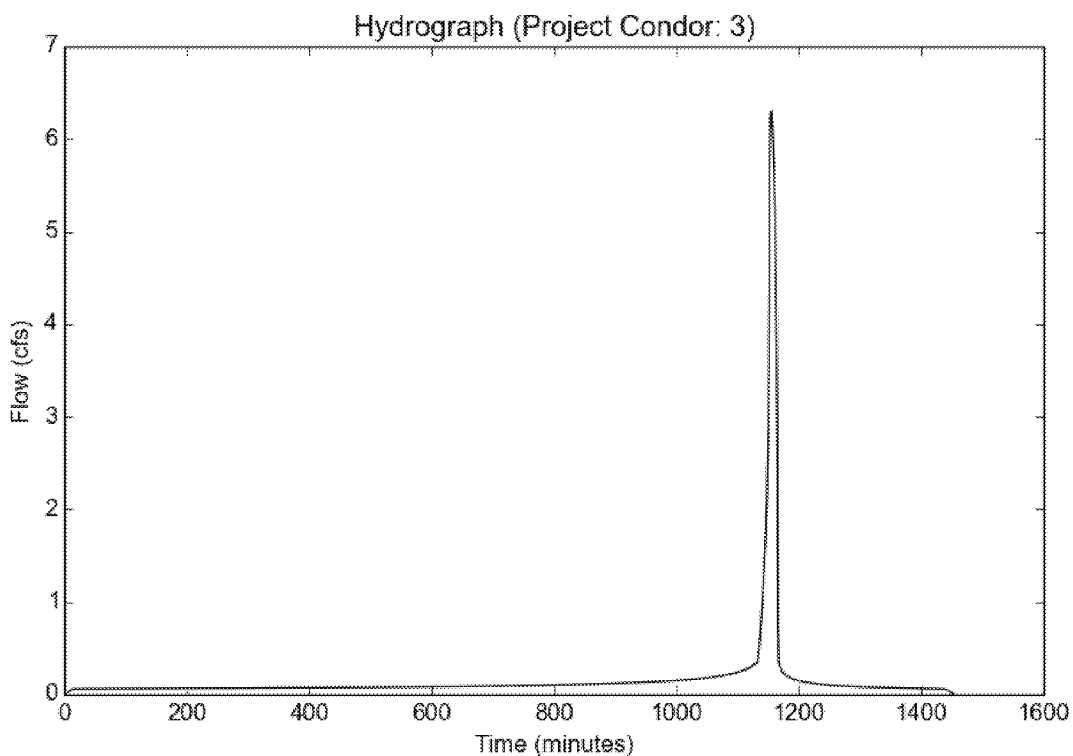
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Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	3
Area (ac)	5.1
Flow Path Length (ft)	685.0
Flow Path Slope (vft/hft)	0.004
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.02
Soil Type	13
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.5217
Peak Intensity (in/hr)	1.6628
Undeveloped Runoff Coefficient (Cu)	0.7394
Developed Runoff Coefficient (Cd)	0.7426
Time of Concentration (min)	14.0
Clear Peak Flow Rate (cfs)	6.2979
Burned Peak Flow Rate (cfs)	6.2979
24-Hr Clear Runoff Volume (ac-ft)	0.3313
24-Hr Clear Runoff Volume (cu-ft)	14431.2809



Peak Flow Hydrologic Analysis

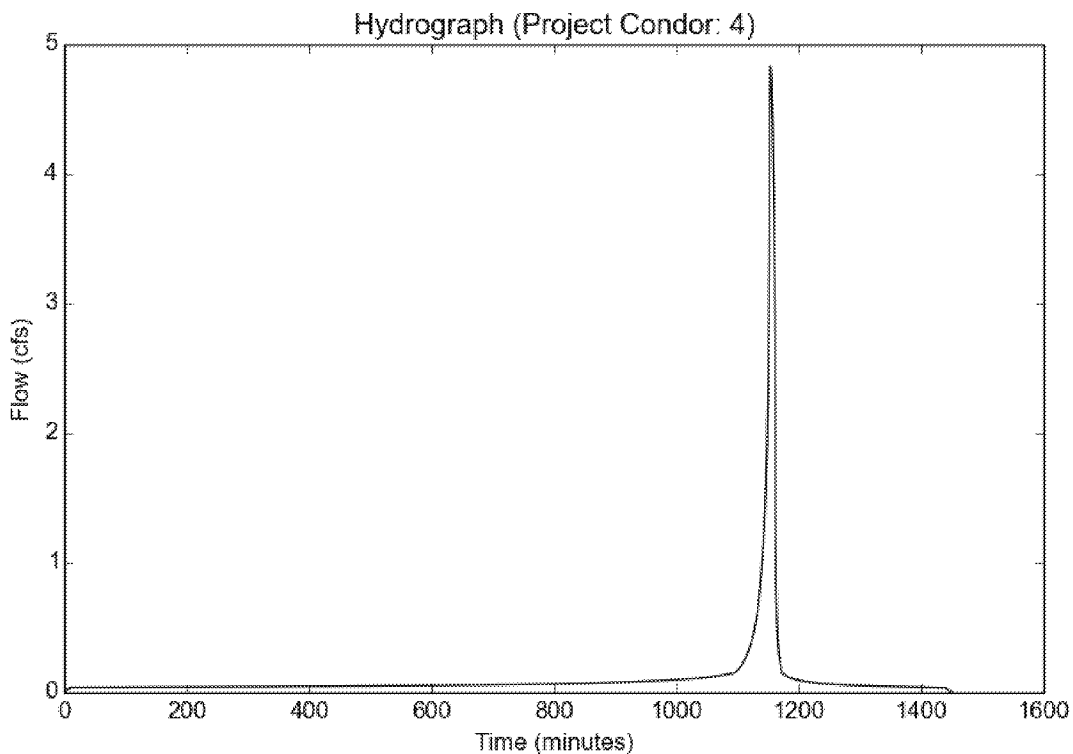
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Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	4
Area (ac)	3.3
Flow Path Length (ft)	427.0
Flow Path Slope (vft/hft)	0.0047
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.02
Soil Type	16
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.5217
Peak Intensity (in/hr)	1.9477
Undeveloped Runoff Coefficient (Cu)	0.749
Developed Runoff Coefficient (Cd)	0.752
Time of Concentration (min)	10.0
Clear Peak Flow Rate (cfs)	4.8336
Burned Peak Flow Rate (cfs)	4.8336
24-Hr Clear Runoff Volume (ac-ft)	0.2283
24-Hr Clear Runoff Volume (cu-ft)	9945.7762



Peak Flow Hydrologic Analysis

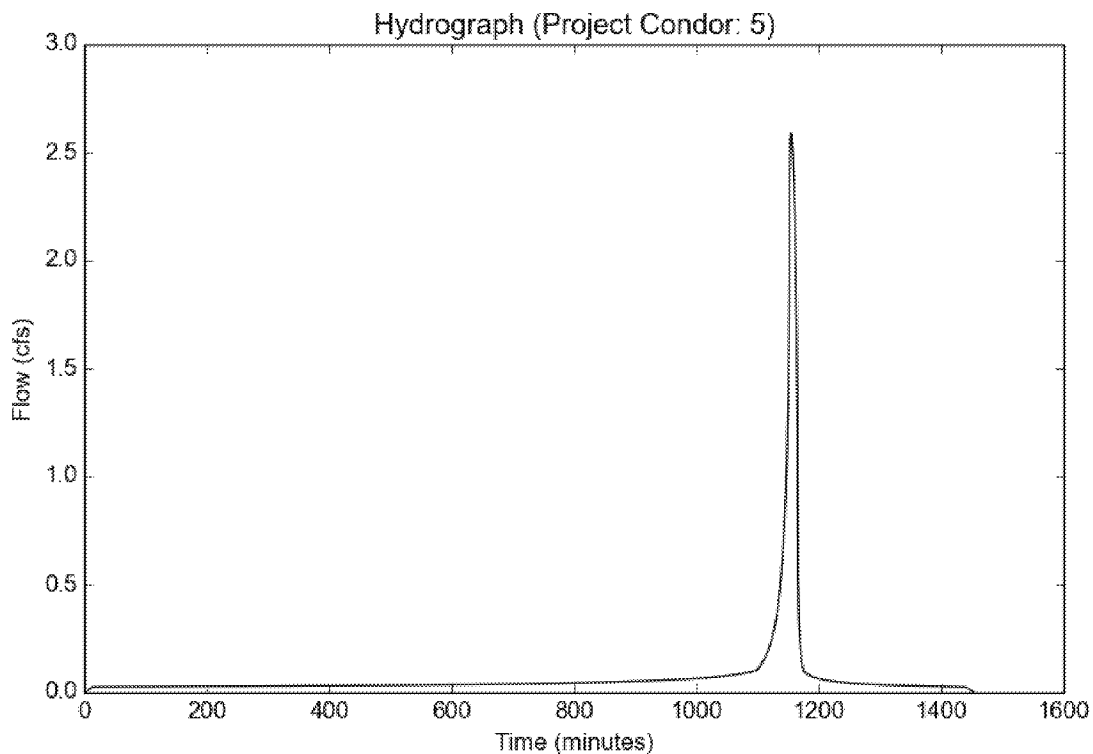
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Version: HydroCalc 1.0.2

Input Parameters

Project Name	Project Condor
Subarea ID	5
Area (ac)	2.2
Flow Path Length (ft)	600.0
Flow Path Slope (vft/hft)	0.003
50-yr Rainfall Depth (in)	5.15
Percent Impervious	0.02
Soil Type	16
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.5217
Peak Intensity (in/hr)	1.6628
Undeveloped Runoff Coefficient (Cu)	0.7037
Developed Runoff Coefficient (Cd)	0.7076
Time of Concentration (min)	14.0
Clear Peak Flow Rate (cfs)	2.5886
Burned Peak Flow Rate (cfs)	2.5886
24-Hr Clear Runoff Volume (ac-ft)	0.1516
24-Hr Clear Runoff Volume (cu-ft)	6601.9462



EXISTING STORM DRAIN LINE	CONTRIBUTING ON-SITE DRAINAGE AREA	ALLOWABLE DISCHARGE TO EXISTING LINE (cfs)	CONTRIBUTING FLOW (cfs)	NOTE
PROJECT DDI #0008	A1 & A2	5.10	4.8	SUFFICIENT SD CAPACITY
PROJECT 4402 LINE C	A6	2.40	2.2	SUFFICIENT SD CAPACITY
PROJECT 681	A3, A4 & A5	16.90	16.6*	SUFFICIENT SD CAPACITY

* TOTAL RUNOFF FROM DRAINAGE AREAS A3 & A4 DETENTION FACILITY OUTFALL OF 7.9 CFS AND DRAINAGE AREA A6 RUNOFF OF 8.7 CFS.



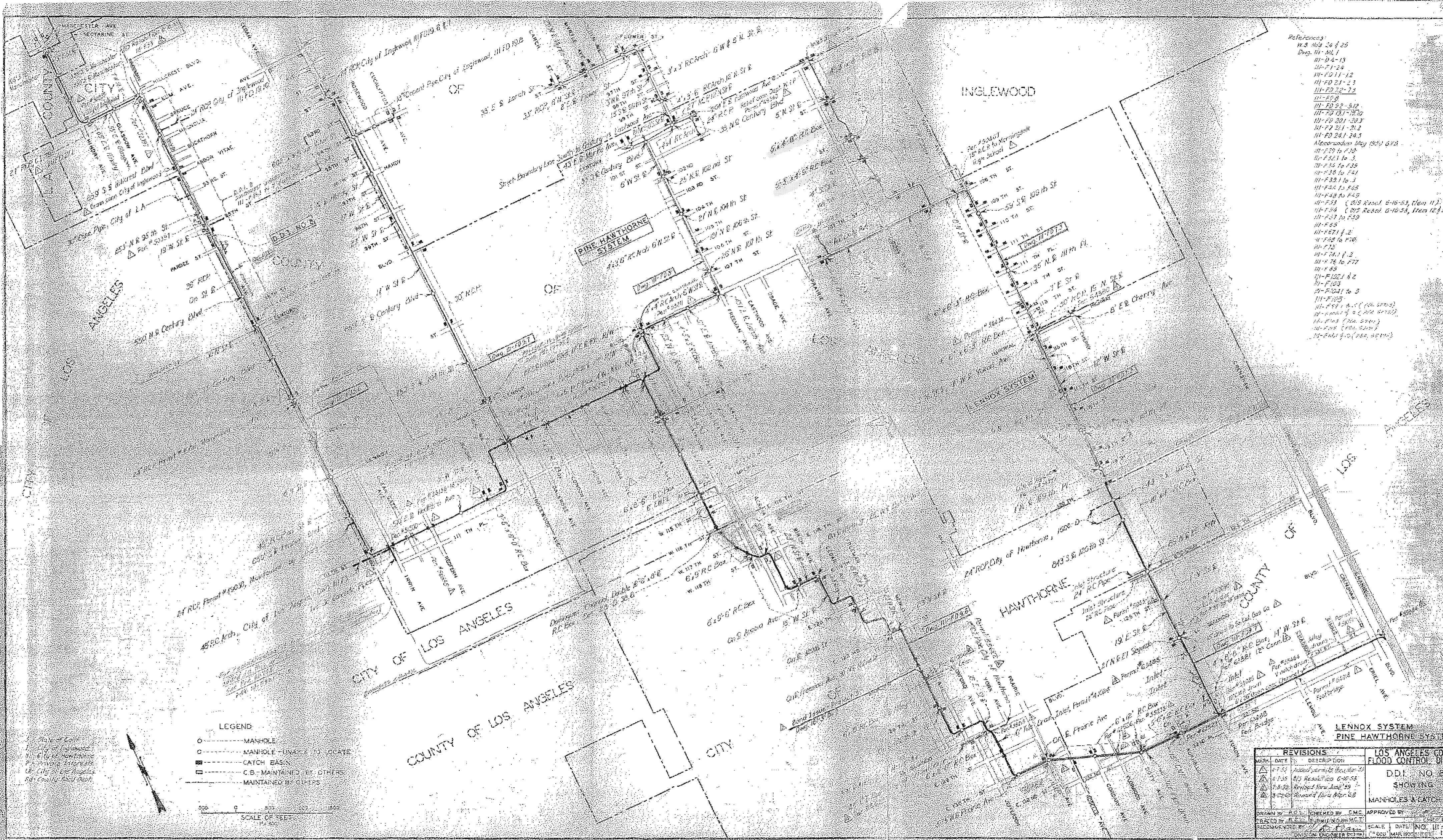
Appendix E

LA County As-Built Plans

LACDPW Project 4402 & 681 As Built Plans

LACDPW Project 4401, Line A As Built Plans

LACDPW DDI # 8 As Built Plans



- References:
- W.S. 103 24 & 25
 - Eng. 11-141
 - 11-14-13
 - 11-14-24
 - 11-FD 11-12
 - 11-FD 21-23
 - 11-FD 22-23
 - 11-25-8
 - 11-FD 22-312
 - 11-FD 131-137
 - 11-FD 201-203
 - 11-FD 211-212
 - 11-FD 241-243
 - Memorandum May 1951 673
 - 11-FD 16 713
 - 11-FD 11 to 3
 - 11-F 16 to F 25
 - 11-F 18 to F 41
 - 11-F 33 to 3
 - 11-F 44 to F 45
 - 11-F 48 to F 49
 - 11-F 51 (City Recd. 8-16-53, Item 11)
 - 11-F 54 (City Recd. 8-16-53, Item 12 & 13)
 - 11-F 57 to F 59
 - 11-F 69
 - 11-F 71 & 2
 - 11-F 78 to F 70
 - 11-F 12
 - 11-F 74 & 2
 - 11-F 76 to F 77
 - 11-F 89
 - 11-F 122 & 2
 - 11-F 103
 - 11-F 121 to 3
 - 11-F 105
 - 11-F 53 & 5 (10-19-53)
 - 11-F 101 & 2 (10-19-53)
 - 11-F 103 (10-19-53)
 - 11-F 105 (10-19-53)
 - 11-F 101 & 2 (10-19-53)

LEGEND

- MANHOLE
- MANHOLE - UNABLE TO LOCATE
- ▣ CATCH BASIN
- ▣ C.B. - MAINTAINED BY OTHERS
- MAINTAINED BY OTHERS

SCALE OF FEET

0 500 1000 1500

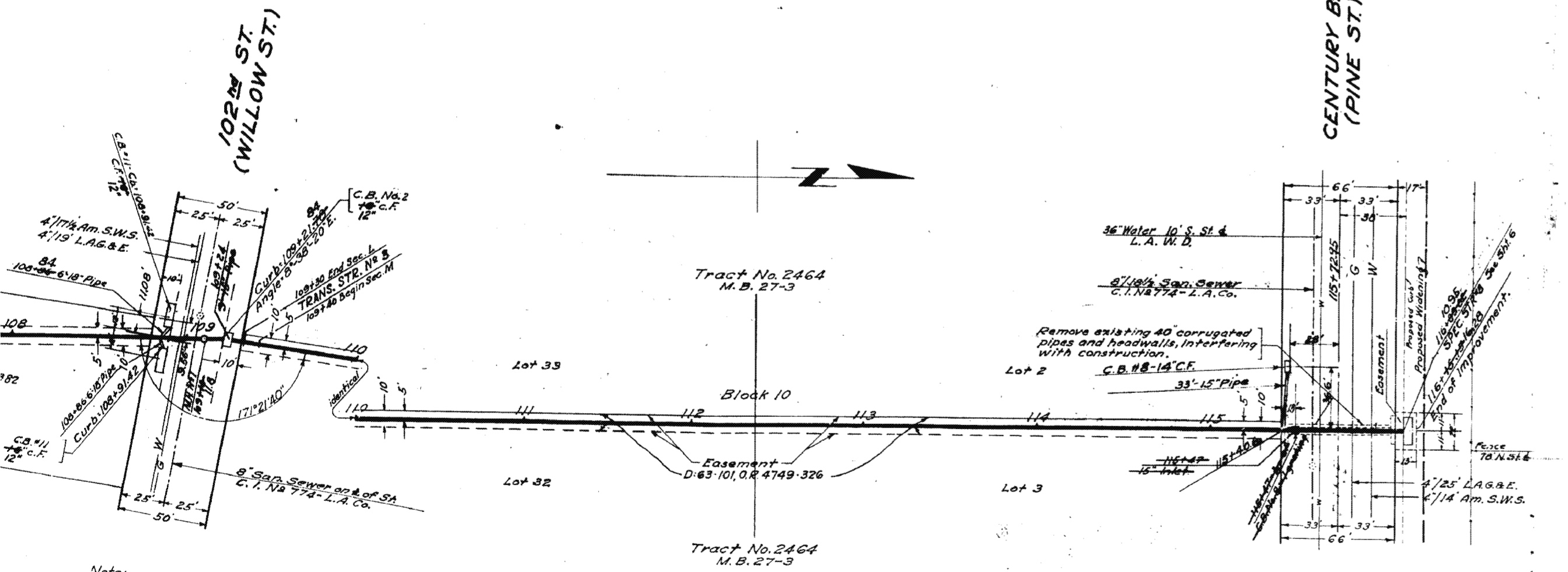
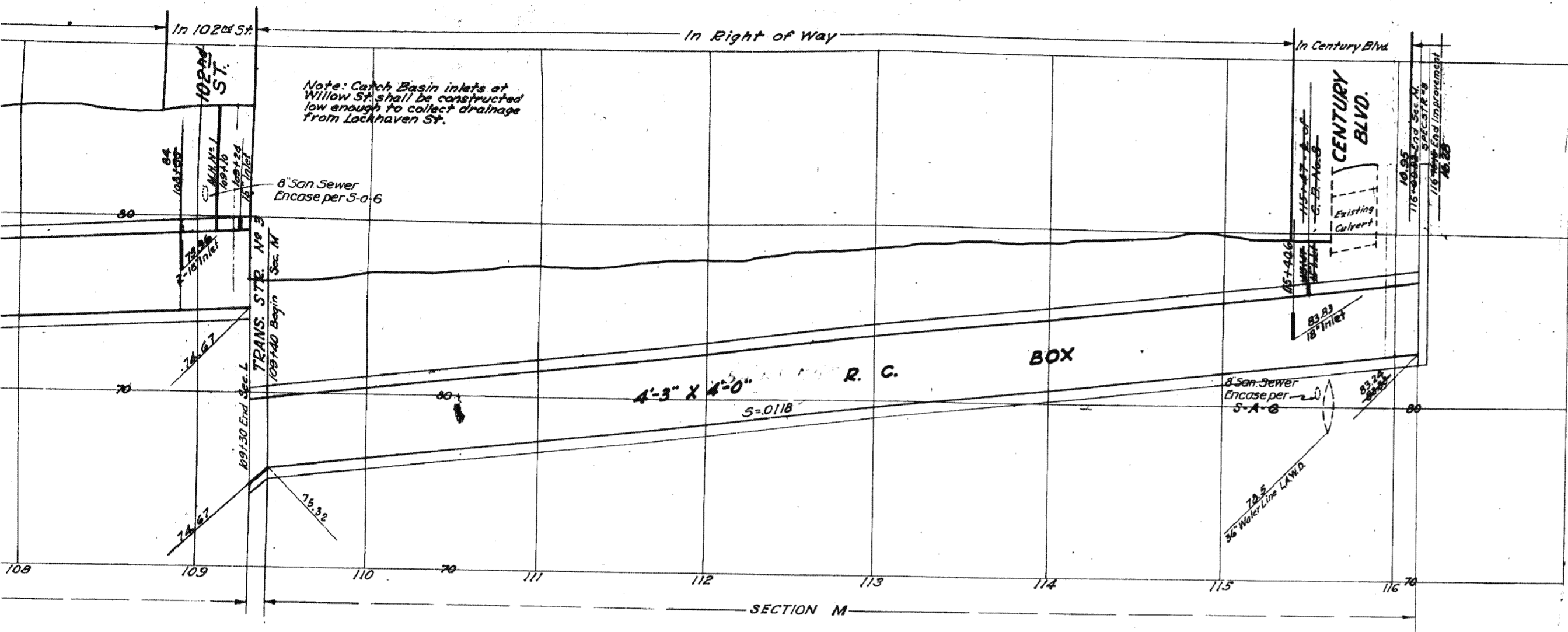
REVISIONS		DESCRIPTION
NO.	DATE	
1	7-7-53	Added permit to the plan
2	7-1-53	815 Resolution 6-10-53
3	7-8-53	Revised from June '53
4	8-27-53	Revised from April '53

LOS ANGELES COUNTY
FLOOD CONTROL DISTRICT

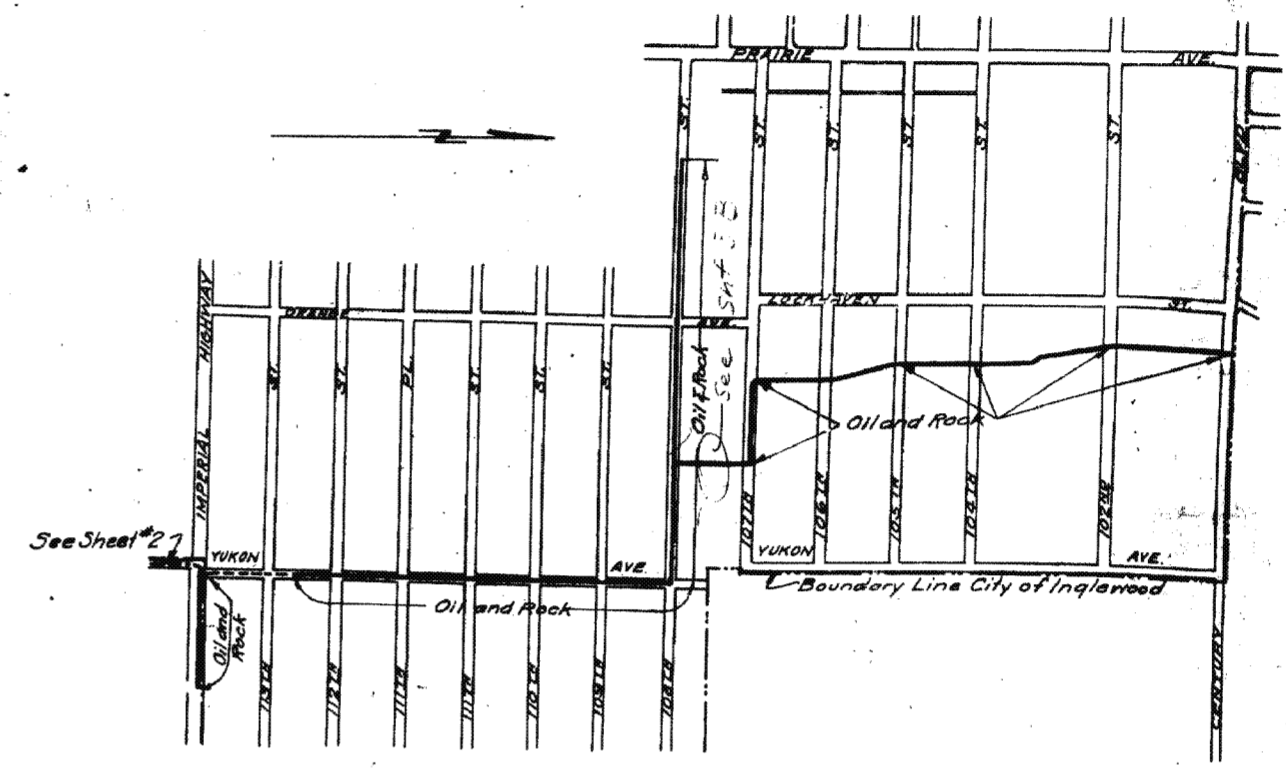
DDI NO. 8
SHOWING
MANHOLES & CATCH BASINS

DRAWN BY: [Name] CHECKED BY: CMC APPROVED BY: [Name]

SCALE: DATE: NO. 11-1-53



LINE



KEY MAP
SHOWING DRAINS AND RESURFACING

111-FD 73

NOTES:

- Elevations are in feet above U.S.G.S. or mean Sea Level Datum.
- Stations of inlets and junctions refer to the intersections of the center lines of the inlets with the center line of the main drain.
- Elevations of all inlets except catch basin inlets into box sections refer to the flow lines of the inlets produced on their gradients to the vertical plane thru the center line of the main drain. Elevations of catch basin inlets into box sections refer to the intersections of the flow lines of these inlets with the inside wall of the box section.
- For details of all standard structures see Standard Plans.
- All pipe not otherwise specified shall be plain cement pipe.
- All stubs to be one length of pipe with ends sealed with 2" red wood planking, unless otherwise specified.
- Catch basins shall be built with a curb face of 10" or more which shall include a minimum depression at opening of 4" below gutter grade, unless otherwise specified.
- Sanitary sewers crossing over or with a clearance of 150 feet or less under Storm Drain, shall be constructed or reconstructed as shown on Standard D-58.
- All reinforced concrete structures shall be of Class "A-1" Concrete.
- Weep holes 3/4" in diameter shall be placed in each sidewall and spaced not more than five feet each way.

LENOX DRAINAGE SYSTEM
OF THE COUNTY OF LOS ANGELES
PLAN AND SPECIFICATIONS
YUKON AVENUE LINE
108TH. ST. BRANCH
IMPERIAL HWY. BRANCH

SHEET 3 OF 10 SHEETS

SCALE: VERT. 1" = 4'
NOVEMBER 1936

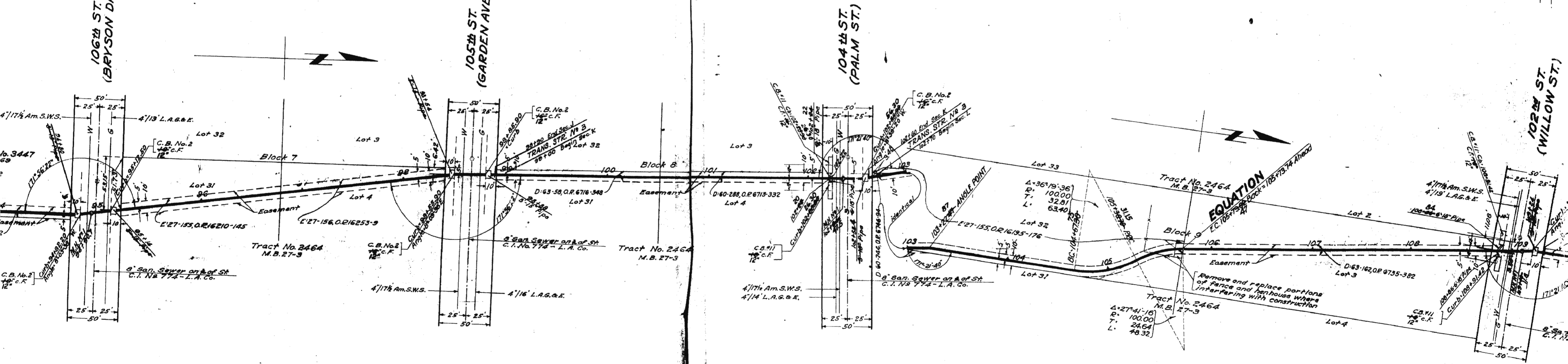
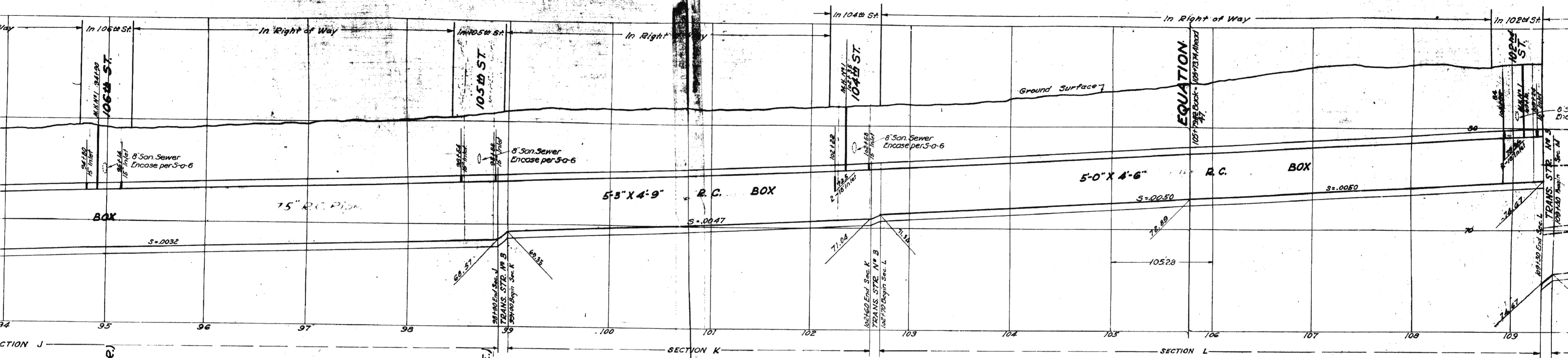
ALFRED JONES COUNTY SURVEYOR

RECOMMENDED: *Edmund* CHIEF DEPUTY
APPROVED: *Alfred Jones* COUNTY SURVEYOR

Corrected as constructed by A.W. Lynn 8-7-42
Checked by L.O. Larphore 8-11-42

REFERENCE		DESIGNED	J.H. Hartley
F. B.	228		
L. B.	289		
Yukon Ave. Line	Revised 6-7-37	TRACED	R.C. Glenn
	7-25-38	CHECKED	J.H. Rice
	10-31-38		

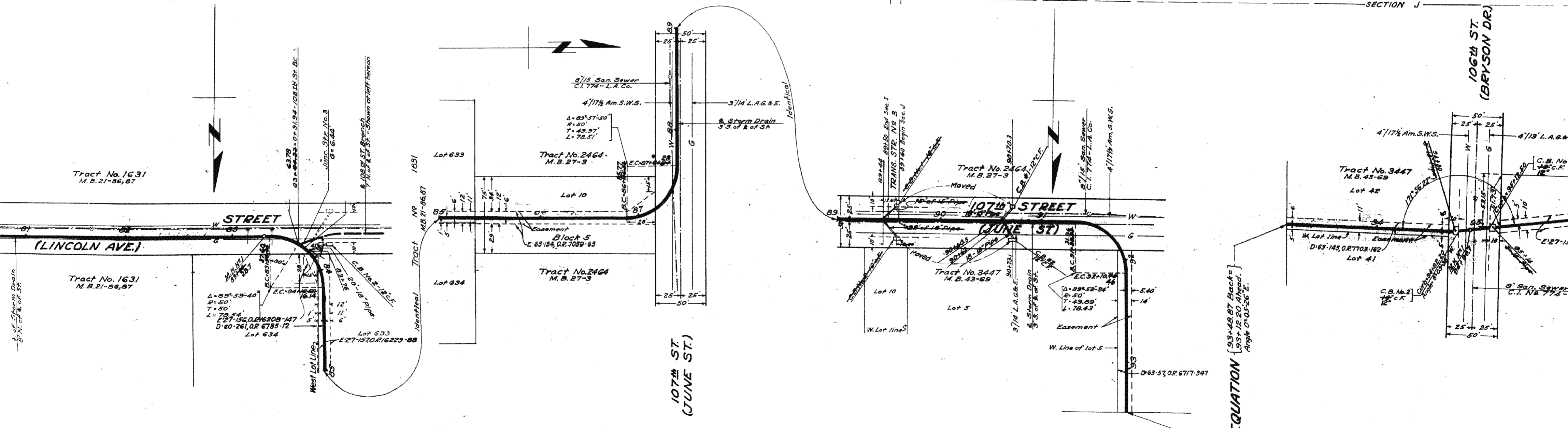
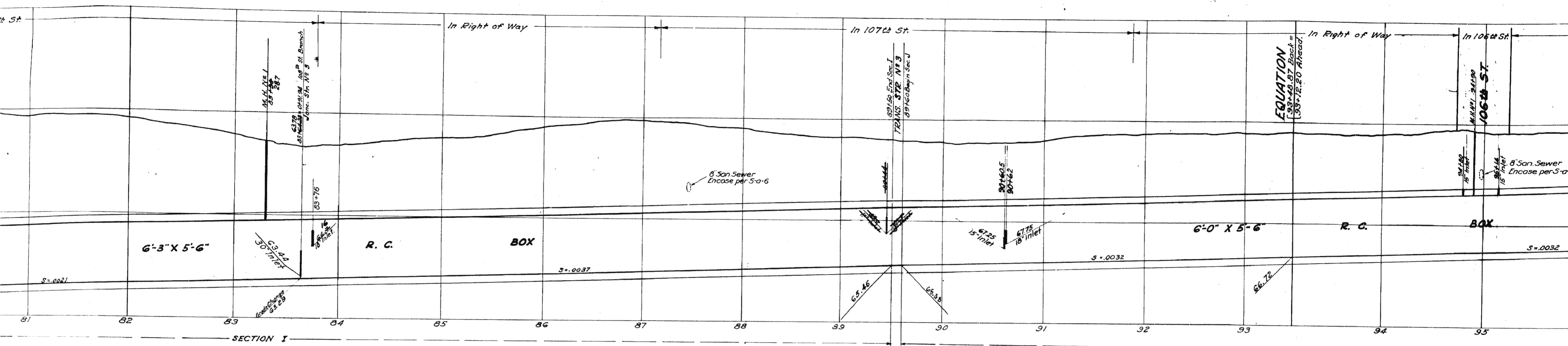
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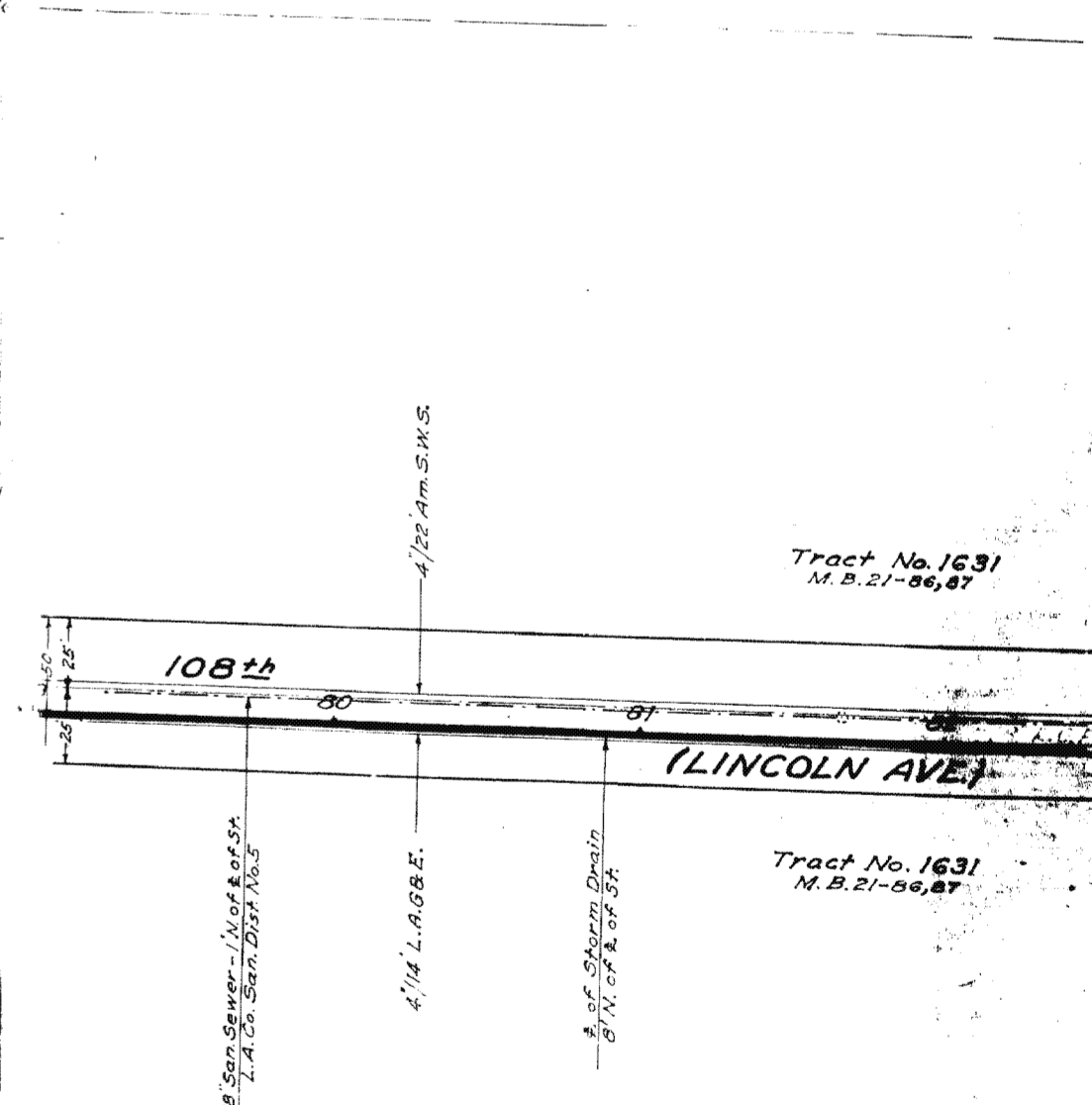
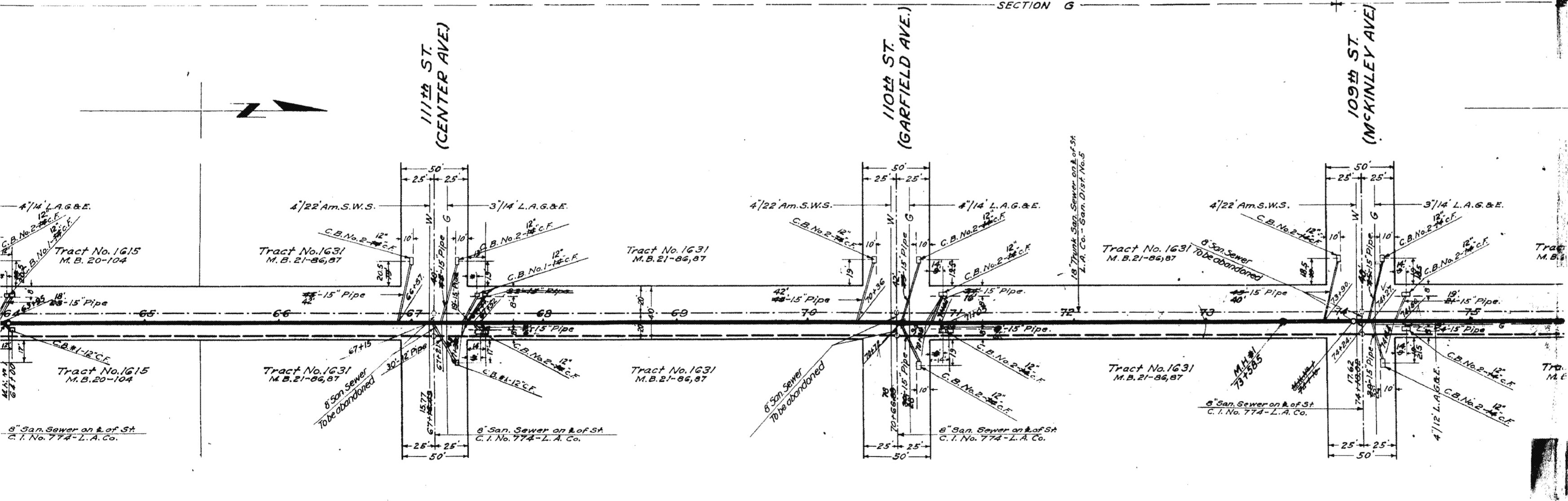
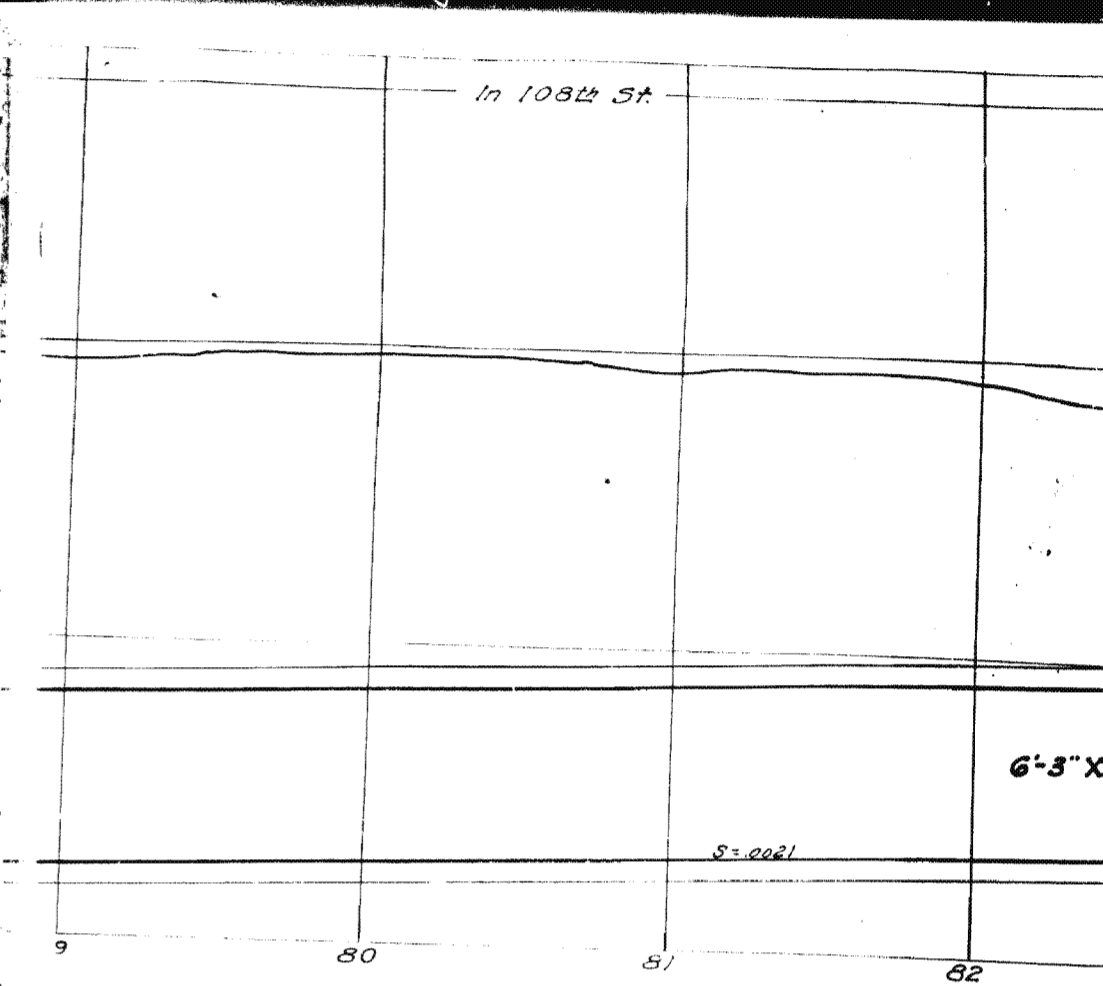
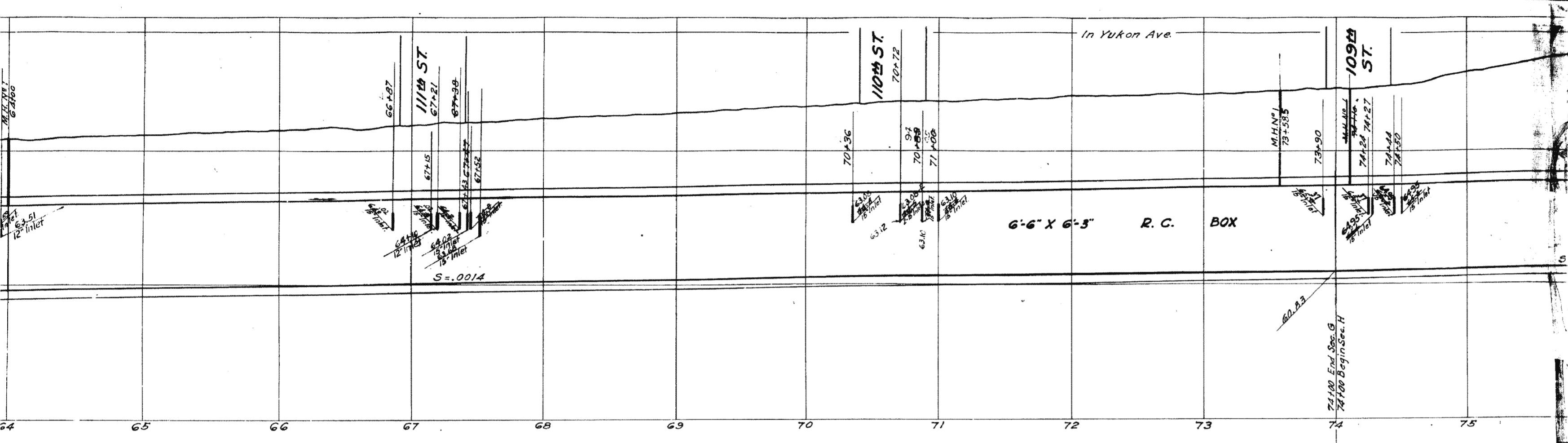
Note:
Elevations for top of curb at catch basins
to be determined in field, making top of curb
4' higher than crown of street.

Note:
Elevations for top of curb to
in field, making top of curb
4' higher than crown of street.

YUKON AVENUE



KON AVENUE LINE



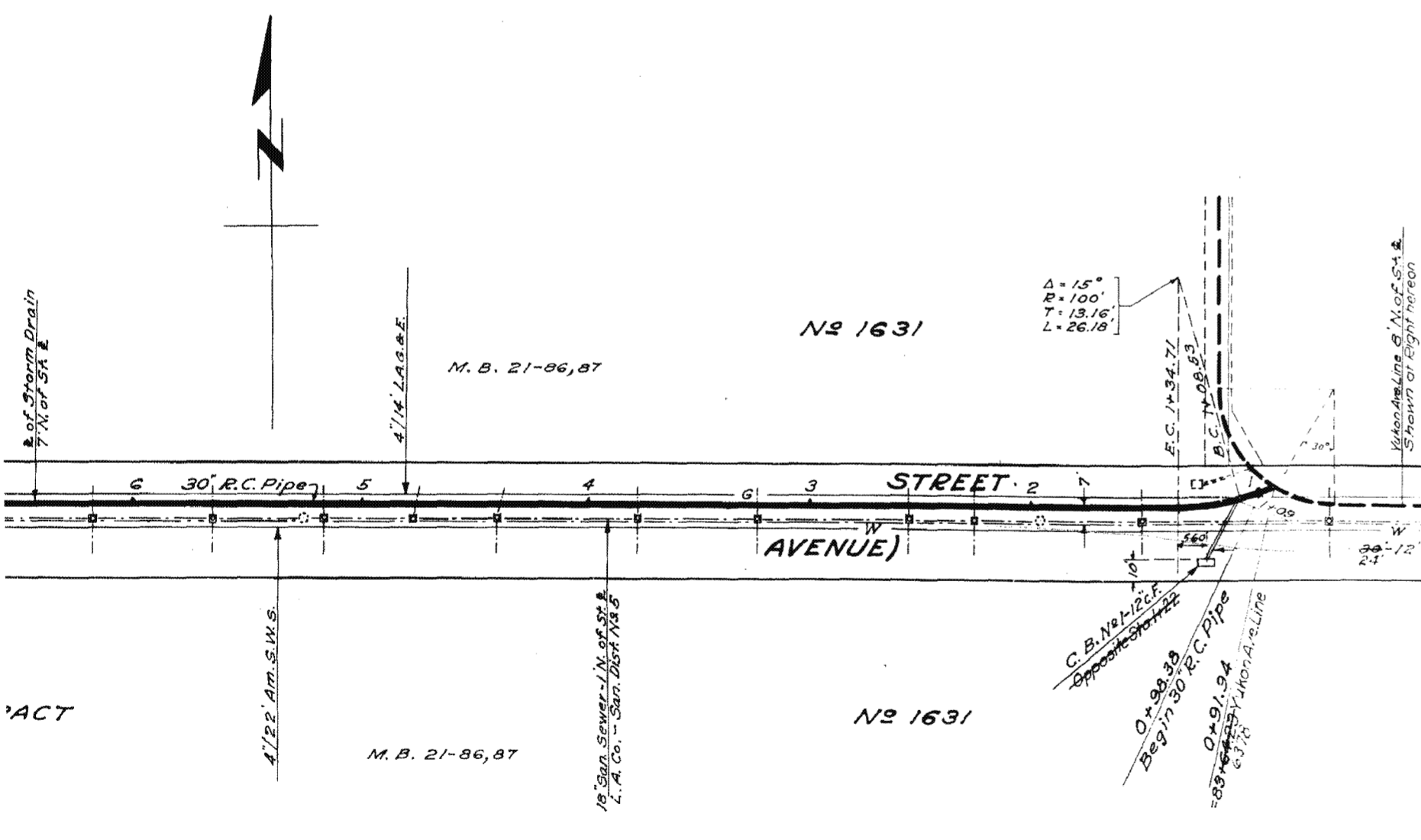
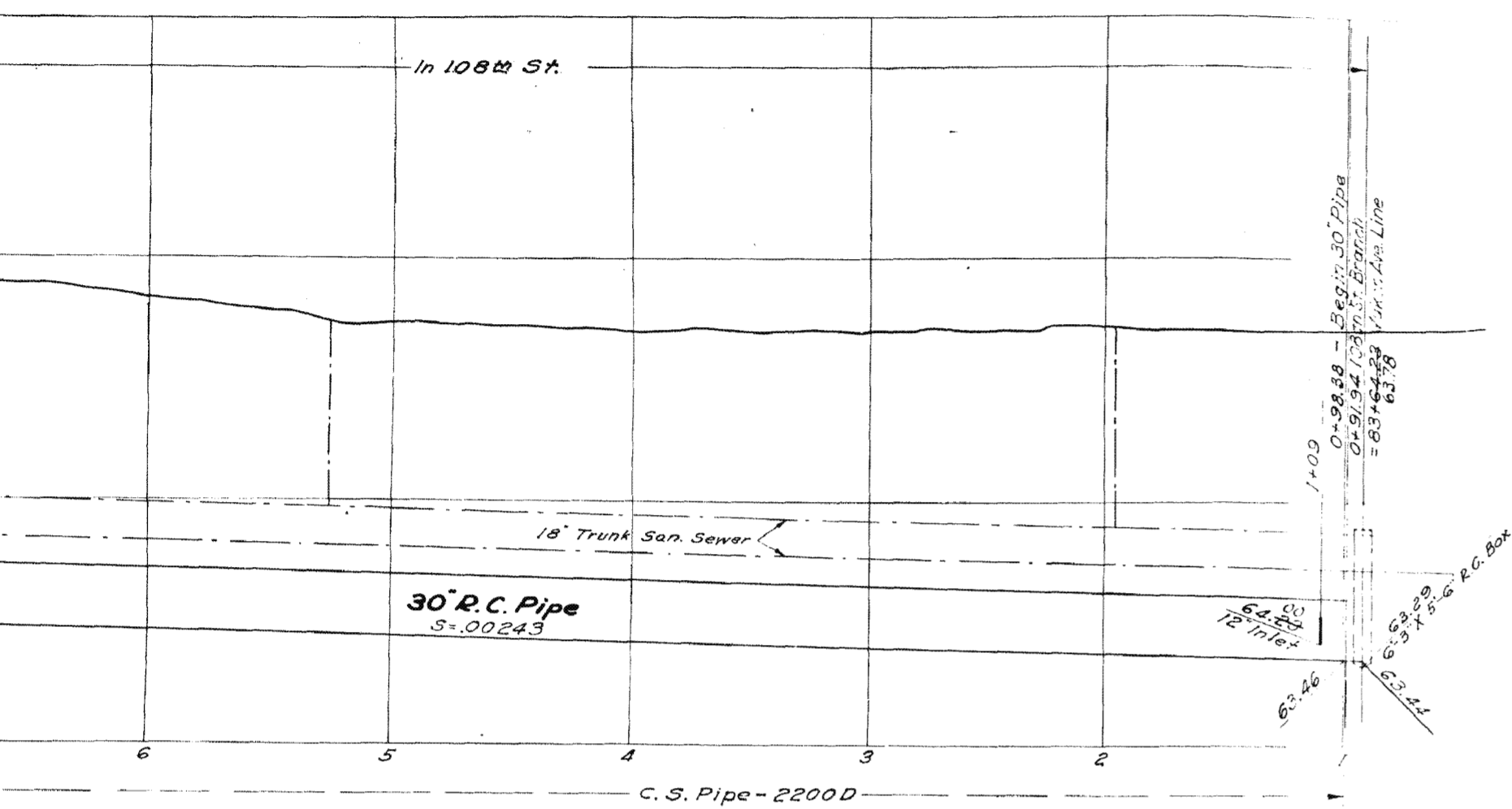
Note:
Elevations for top of curb of catch basins
to be determined in field, making top of curb
4" higher than crown of street.

LINE

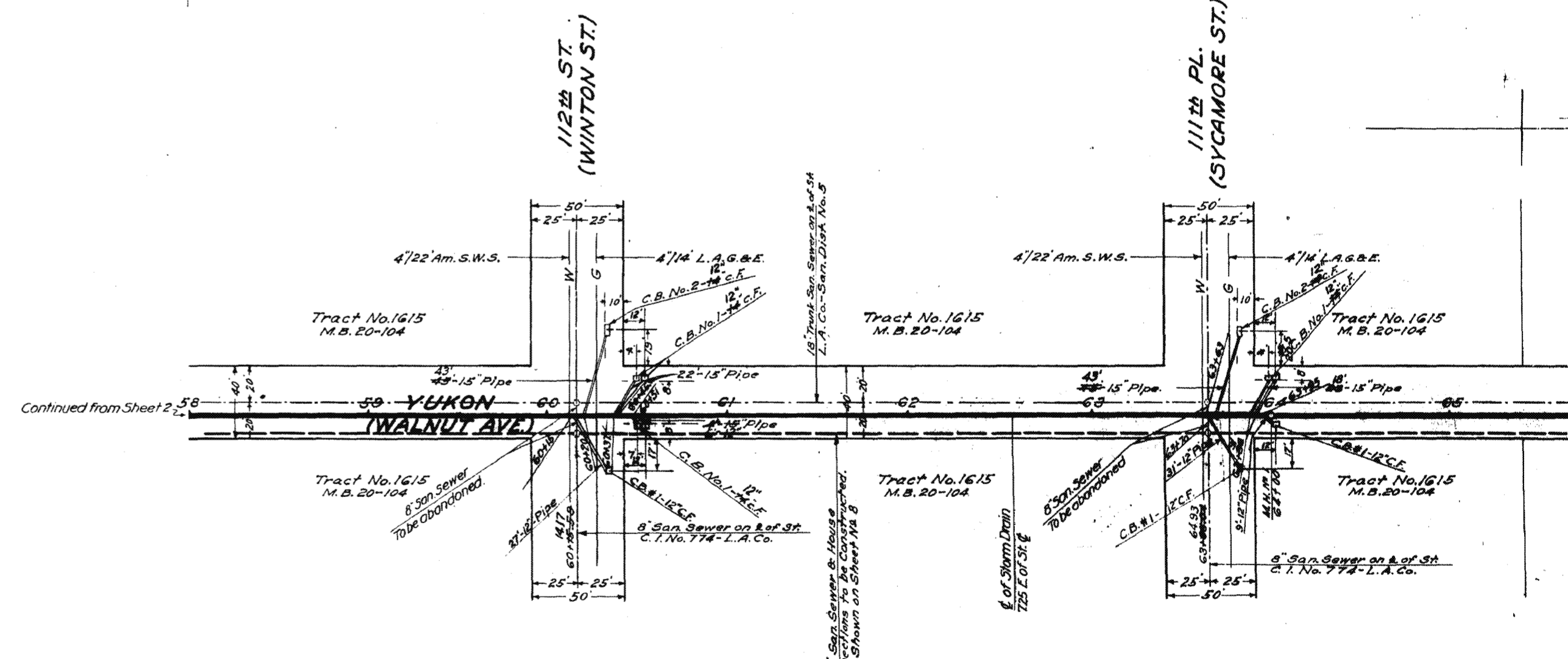
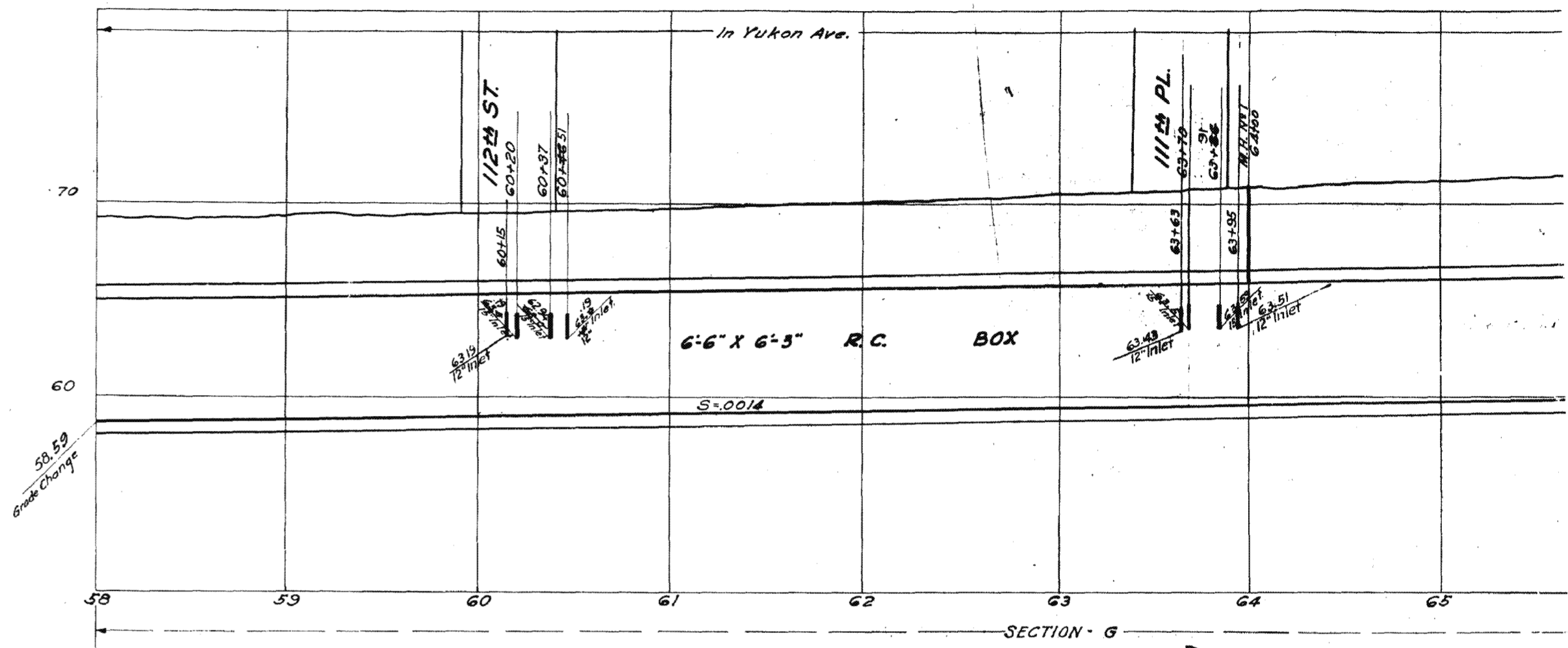
YUKON

AVE

F4



BRANCH

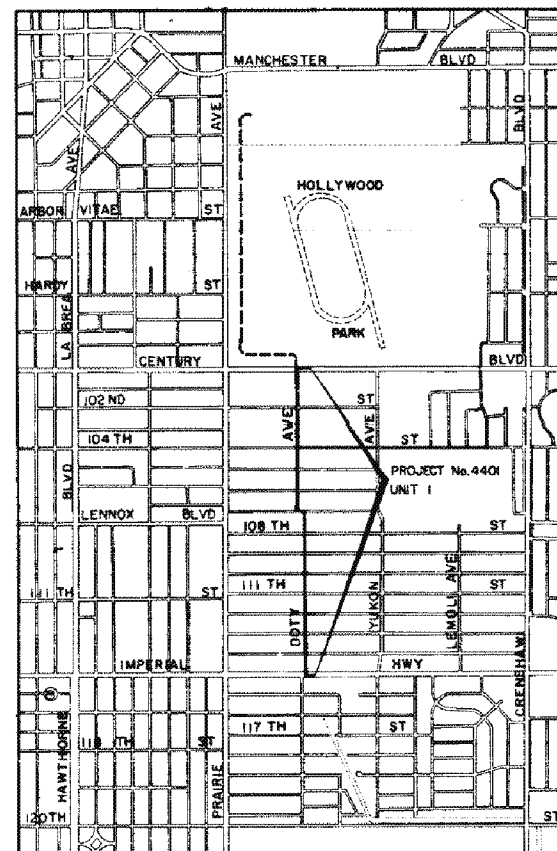


YUKON

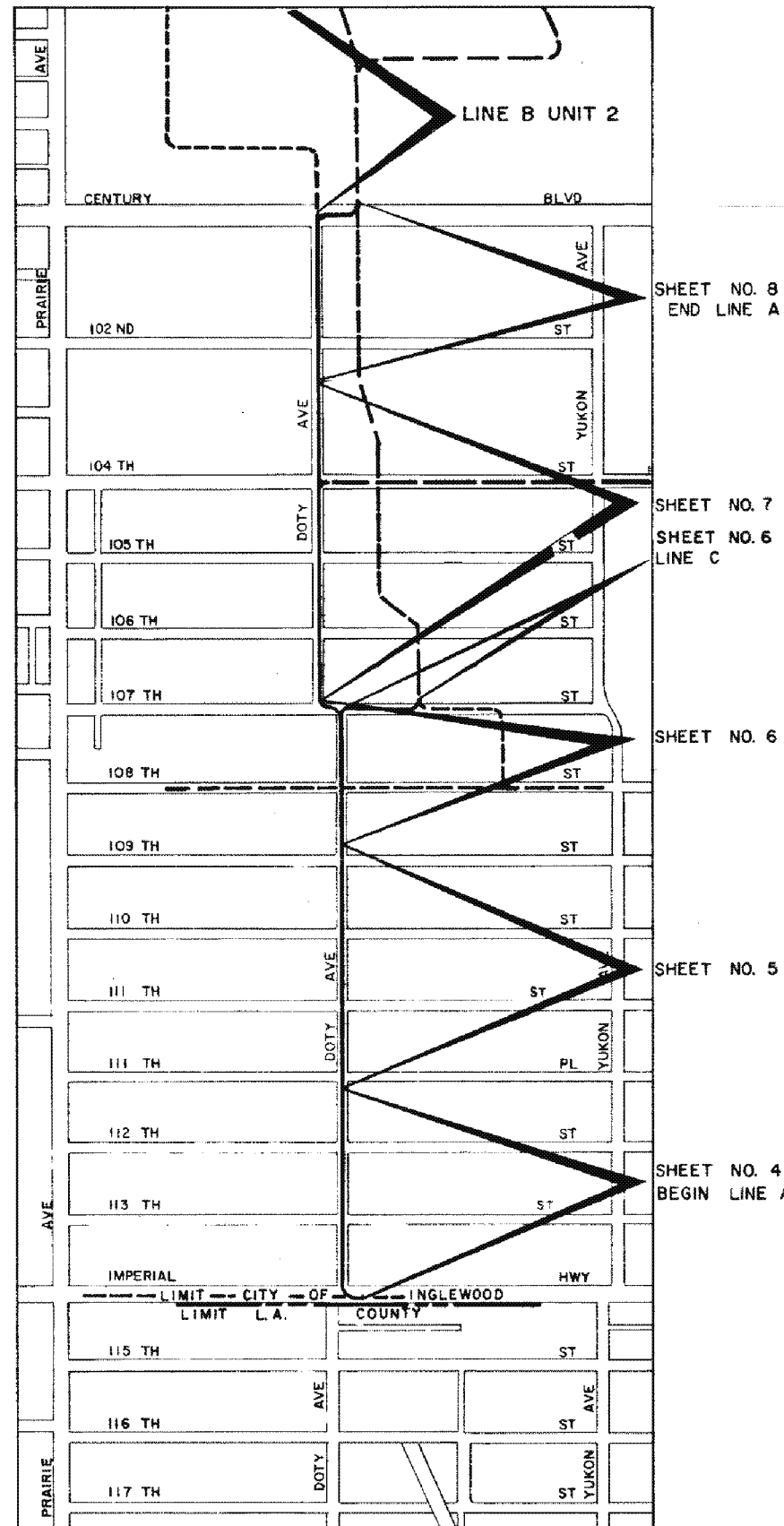
AVENUE

WILLIAMS

Note:
Elevations for top of curb at catch basins
to be determined in field, making top of curb
4" higher than street crown.



VICINITY MAP
1000 0 1000 2000 3000 4000
GRAPHIC SCALE



LOCATION MAP
500 0 500 1000 1500
GRAPHIC SCALE

INDEX TO DRAWINGS

SHEET NO.	TITLE
1.	VICINITY MAP, LOCATION PLAN, INDEX TO DRAWINGS
2.	GENERAL NOTES, STRUCTURAL NOTES
3.	LEGEND & ABBREVIATIONS, STANDARD DRAWINGS, STRUCTURAL NOTES
4.	PLAN & PROFILE CONCRETE CONDUIT LINE A
5.	PLAN & PROFILE CONCRETE CONDUIT LINE A
6.	PLAN & PROFILE CONCRETE CONDUIT LINES A AND C
7.	PLAN & PROFILE CONCRETE CONDUIT LINE A
8.	PLAN & PROFILE CONCRETE CONDUIT LINE A
9.	PLAN & PROFILE COUNTY TRUNK LINE, SEWER LINE A & SEWER SIPHON
10.	PLAN & PROFILE SANITARY SEWER SIPHON
11.	SINGLE R.C. BOX SCHEDULE & DETAILS
12.	SINGLE R.C. BOX SCHEDULE
13.	RESURFACING PLAN
14.	LOG OF BORINGS

■ "AS BUILT" DWGS.

LEGEND

- Proposed Storm Drain (UNIT I)
- Proposed Storm Drain
- Existing Storm Drain

REVISIONS		
MARK	DATE	DESCRIPTION



CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		1964 STORM DRAIN BOND ISSUE LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: <i>William M. Baker</i> ASSISTANT CITY ENGINEER R.C.E. 17167		PROJECT NO. 4401 INGLEWOOD LINES A AND C UNIT I VICINITY MAP, LOCATION MAP AND INDEX TO DRAWINGS	
APPROVED BY: <i>W. E. Dunington</i> CITY ENGINEER R.C.E. 11237		RECOMMENDED BY: <i>R. J. Milne</i> DIVISION ENGINEER (DESIGN) 12-3-74	
SCALE AS SHOWN	DATE Sep 74	DWG. NO. 364-440-D6.1 SHEET 1 OF 14	

"AS BUILT" DRAWING

GENERAL NOTES

1. NUMBERS IN CIRCLES INDICATE ITEM UNDER WHICH PAYMENT WILL BE MADE.
2. ELEVATIONS SHOWN ARE IN FEET ABOVE THE U.S.G.S. MEAN SEA LEVEL DATUM.
3. STATIONS SHOWN ON DRAWINGS ARE ALONG CENTERLINE OF CONDUIT OR ON A LINE NORMAL TO CENTERLINE OF CONDUIT.
4. STATIONS AND INVERT ELEVATIONS OF PIPE INLETS SHOWN ON THE PROFILES ARE AT THE INSIDE FACE OF THE CONDUIT, UNLESS OTHERWISE SHOWN.
5. ALL FIELD BOOK REFERENCES ARE TO CITY OF INGLEWOOD FIELD BOOKS, UNLESS OTHERWISE NOTED.
6. LOCATIONS SHOWN ON THE PLANS FOR EXISTING SANITARY SEWER HOUSE CONNECTIONS ARE APPROXIMATE ONLY.
7. PIPE CONNECTIONS TO STORM DRAIN SHALL CONFORM TO STANDARD DRAWING 2-D191 & 2-D193 UNLESS OTHERWISE SHOWN.
8. TIES FOR CATCH BASINS AS SHOWN ON THE DRAWINGS ARE FROM CURB RETURN TO CENTER LINE OF CATCH BASINS, UNLESS OTHERWISE SHOWN.
9. LOCATIONS OF CATCH BASIN CONNECTOR PIPE JUNCTIONS WITH CATCH BASINS AS SHOWN ON THE DRAWINGS ARE SCHEMATIC. IT IS INTENDED THAT SUCH JUNCTIONS BE LOCATED AT THE DOWNSTREAM ENDS OF THE CATCH BASINS, UNLESS A DETAIL OF THE CONNECTION IS SHOWN OR A NOTE SPECIFICALLY INDICATES OTHERWISE. IN ALL CASES THE EXACT LOCATIONS WILL BE DETERMINED IN THE FIELD BY THE ENGINEER TO MEET FIELD CONDITIONS.
10. MONOLITHIC CATCH BASIN CONNECTIONS SHALL BE CONSTRUCTED, WHERE APPLICABLE, PER STANDARD DRAWING 2-D224.
11. "V₁" IS THE DEPTH OF INLET OF CATCH BASINS IN SERIES MEASURED FROM TOP OF CURB TO INVERT OF CONNECTOR PIPE. "V₂" IS THE SAME AS "V" ON THE VARIOUS CATCH BASIN STANDARD DRAWINGS.
12. ALL EXISTING SANITARY SEWERS SHOWN ON THE DRAWINGS ARE CITY OF INGLEWOOD SEWERS, UNLESS OTHERWISE NOTED.
13. EXISTING UTILITIES SHALL BE MAINTAINED IN PLACE BY THE CONTRACTOR, UNLESS OTHERWISE NOTED.
14. DELETED.
15. UTILITIES DESIGNATED BY THE SYMBOL "*" WILL BE ABANDONED IN PLACE AND THE OWNER WILL INSTALL A NEW SECTION OF THE AFFECTED UTILITY AT A LOCATION IN CLOSE PROXIMITY TO, BUT WHICH DOES NOT PHYSICALLY INTERFERE WITH, THE PROPOSED STORM DRAIN CONDUIT AND APPURTENANT STRUCTURES.
16. WHERE UTILITIES ARE INDICATED ON THE DRAWINGS TO BE SUPPORTED, SAID SUPPORTS SHALL BE IN ACCORDANCE WITH STANDARD DRAWING 2-D173.1, .2 OR .3, UNLESS OTHERWISE INDICATED.

GENERAL NOTES

17. SANITARY SEWER HOUSE CONNECTION RECONSTRUCTION AND RECONNECTION SHALL BE IN ACCORDANCE WITH STANDARD DRAWING 2-D250, UNLESS OTHERWISE SHOWN.
18. SANITARY SEWERS AND HOUSE CONNECTIONS CROSSING OVER THE STORM DRAIN TRENCH SHALL BE SUPPORTED IN ACCORDANCE WITH STANDARD DRAWING 2-D173.1 TO .3 AND ENCASED PER GENERAL NOTE 1 ON STANDARD DRAWING 2-D173.1.
19. WHEN INDICATED ON THE DRAWINGS, SANITARY SEWERS AND HOUSE CONNECTIONS SHALL BE ENCASED OR BLANKETED IN ACCORDANCE WITH STANDARD DRAWING 2-D251.
20. ALL OPENINGS RESULTING FROM THE CUTTING OR PARTIAL REMOVAL OF EXISTING CULVERTS, PIPES OR SIMILAR STRUCTURES SHALL BE SEALED WITH 8 INCHES OF BRICK AND MORTAR OR 6 INCHES OF CONCRETE, UNLESS OTHERWISE SHOWN.
21. ALL RESURFACING, CURBS, GUTTERS, SIDEWALKS, DRIVEWAYS AND OTHER EXISTING IMPROVEMENTS TO BE RECONSTRUCTED SHALL BE CONSTRUCTED AT THE SAME ELEVATION AND LOCATION AS THE EXISTING IMPROVEMENTS, UNLESS OTHERWISE NOTED.
22. REFER TO SHEET 13 FOR TYPICAL CATCH BASIN CONNECTOR PIPE PROFILE.
23. STREET RESURFACING PLANS ARE SHOWN ON SHEET 13.
24. SOIL TEST BORINGS FOR THIS PROJECT WERE MADE IN AUGUST AND SEPTEMBER, 1972
25. PAY LINES FOR EXCAVATION FOR REINFORCED CONCRETE BOX CONDUIT ARE SHOWN ON SHEET 11 OF THE DRAWINGS.
26. DESIGN OF THE PIPE SHOWN HEREON IS BASED ON THE ASSUMPTION THE PIPE WILL BE INSTALLED IN ACCORDANCE WITH CASE III BEDDING AS SHOWN ON STANDARD DRAWING 2-D177 UNLESS OTHERWISE SHOWN. "W" VALUES SHALL BE AS SPECIFIED ON STANDARD DWG. 2-D177 FOR CASE III BEDDING, NOTES 3(a), 3(b) AND 3(c). IF THE "W" VALUE AT THE TOP OF THE PIPE IS EXCEEDED, THE BEDDING SHALL BE MODIFIED, AND/OR PIPE OF ADDITIONAL STRENGTH SHALL BE PROVIDED. THE PROPOSED MODIFICATION SHALL BE APPROVED BY THE DISTRICT.
27. CONCRETE BACKFILL SHALL BE PROVIDED AROUND PIPE 21" IN DIAMETER OR LESS WHERE THE COVER IS EQUAL TO OR LESS THAN 2'-0", AROUND PIPE GREATER THAN 21 INCHES IN DIAMETER BUT LESS THAN 39 INCHES WHERE THE COVER IS LESS THAN 1'-3", AND FOR PIPE 39" OR GREATER WHERE THE COVER IS LESS THAN 1'-0". THE CONCRETE BACKFILL SHALL BE AS SPECIFIED ON STANDARD DRAWING 2-D177, NOTE 3.
28. CURB FACE (C.F) SHOWN ON THE GENERAL PLAN PERTAINS TO THE CURB FACE AT THE CATCH BASIN OPENING UNLESS OTHERWISE NOTED. FOR LOCAL DEPRESSION NO. 2, THE CURB FACE SHALL BE THE EXISTING PLUS 4", UNLESS OTHERWISE SPECIFIED.
29. ASBESTOS CEMENT PIPE MAY BE USED IN LIEU OF REINFORCED CONCRETE PIPE 42 INCHES OR LESS IN DIAMETER.
30. THE REQUIRED D-LOAD FOR THE ASBESTOS CEMENT PIPE SHALL BE 1.5 TIMES THAT SPECIFIED ON THE PLANS FOR REINFORCED CONCRETE PIPE.
31. UTILITIES DESIGNATED BY THE SYMBOL # WILL BE REMOVED BY THE OWNER AND THE OWNER WILL REINSTALL A NEW SECTION OF THE AFFECTED UTILITY AT A LOCATION IN CLOSE PROXIMITY TO, BUT WHICH DOES NOT PHYSICALLY INTERFERE WITH, THE PROPOSED STORM DRAIN CONDUIT AND APPURTENANT STRUCTURES.

STRUCTURAL NOTES

1. DIMENSIONS FROM FACE OF CONCRETE TO STEEL ARE TO CENTER OF BAR AND SHALL BE 2" UNLESS OTHERWISE SHOWN.
2. CONCRETE DIMENSIONS SHALL BE MEASURED HORIZONTALLY OR VERTICALLY ON THE PROFILE, AND PARALLEL TO OR AT RIGHT ANGLES (OR RADIALLY) TO CENTERLINE OF CONDUIT ON THE PLAN EXCEPT AS OTHERWISE SHOWN.
3. ALL BAR BENDS AND HOOKS SHALL CONFORM TO THE 1963 AMERICAN CONCRETE INSTITUTE'S "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" SECTION 801.
4. PLACING OF REINFORCEMENT SHALL CONFORM TO THE 1963 AMERICAN CONCRETE INSTITUTE'S "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" SECTION 803.
5. TRANSVERSE CONSTRUCTION JOINTS SHALL NOT BE PLACED WITHIN 30 INCHES OF MANHOLE OR JUNCTION STRUCTURE OPENINGS.
6. TRANSVERSE CONSTRUCTION JOINTS IN WALLS AND SLABS SHALL BE IN THE SAME PLANE. NO STAGGERING OF JOINTS WILL BE PERMITTED. TRANSVERSE CONSTRUCTION JOINTS SHALL BE NORMAL OR RADIAL TO THE CENTER LINE OF CONSTRUCTION.
7. THE TRANSVERSE REINFORCING STEEL SHALL TERMINATE 1-1/2 INCHES FROM THE CONCRETE SURFACE UNLESS OTHERWISE SHOWN ON THE STRUCTUAL DETAILS.
8. EXPOSED EDGES OF CONCRETE MEMBERS SHALL BE ROUNDED OR BEVELED.
9. NO SPLICES IN TRANSVERSE STEEL REINFORCEMENT WILL BE PERMITTED OTHER THAN SHOWN ON THE DRAWING WITHOUT APPROVAL OF THE ENGINEER. NO MORE THAN 2 SPLICES WILL BE PERMITTED IN ANY LONGITUDINAL BAR BETWEEN TRANSVERSE JOINTS. SPLICES SHALL BE STAGGERED.
10. LONGITUDINAL STEEL SHALL BE LAPPED 20 BAR DIAMETERS AT SPLICES. TRANSVERSE STEEL SHALL BE LAPPED 30 BAR DIAMETERS AT SPLICES.
11. LONGITUDINAL STEEL SHALL BE CONTINUOUS AND EXTEND THROUGH ALL CONSTRUCTION JOINTS.
12. UNLESS OTHERWISE SHOWN ON THE DRAWINGS, TRANSVERSE JOINT KEYWAYS (IN BOTH SLABS AND WALLS), AS DETAILED FOR LONGITUDINAL KEYWAYS AT THE BASE OF THE WALLS, SHALL BE PLACED AT THE END OF EACH POUR, BUT THE SPACING THEREOF SHALL NOT EXCEED 50 FEET OR BE LESS THAN 10 FEET. SPACING MAY BE DECREASED TO AVOID PROXIMITY TO INLETS. ALL CONSTRUCTION JOINTS IN BOTTOM SLAB, TOP SLAB, AND SIDE WALLS SHALL BE IN THE SAME PLANE. NO STAGGERING OF JOINTS WILL BE PERMITTED.
13. THE VERTICAL STEEL IN THE INTERIOR FACE OF EXTERIOR WALLS MAY BE SPLICED AT THE CONSTRUCTION JOINT AT THE BASE OF THE WALL. THE SPLICE SHALL BE 20 BAR DIAMETERS IN LENGTH.



1964 STORM DRAIN BOND ISSUE

REVISIONS			
MARK	DATE	DESCRIPTION	

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: <i>William J. Mohr</i> ASSISTANT CITY ENGINEER R.C.E. 17167		PROJECT NO. 4401 INGLEWOOD LINES A AND C UNIT I GENERAL NOTES & STRUCTURAL NOTES	
APPROVED BY: <i>W. J. Lam</i> 1-3-74 CITY ENGINEER R.C.E. 11237		RECOMMENDED BY: <i>Rymul</i> DIVISION ENGINEER (DESIGN)	
SCALE AS SHOWN	DATE Sept, 74	DWG. NO. 364-4401-D6.2	SHEET 2 OF 14

"AS BUILT" DRAWING

LEGEND AND ABBREVIATIONS

RIGHT OF WAY (OR EASEMENT)	
PROPERTY LINE	
CURB	
GUTTER	
SIDE INLET CATCH BASIN	
EXISTING DRAINAGE STRUCTURES	
GRATING TYPE CATCH BASIN	
DRIVEWAY	
LIMIT OF CONCRETE SURFACE	
WALK	
TRAFFIC SIGNAL	
DEAD MAN	
POWER, TELEPHONE OR GUY POLE	
FIRE HYDRANT	
ELECTROLIER	
WATER & GAS METER	
BENCH MARK	
TEST BORING	
TREE	
TREES TO BE REPLACED BY CONTRACTOR	
UNDER EXISTING UTILITY	
OVER EXISTING UTILITY	
TRAFFIC SIGNAL CONDUIT	T.S.C.
TRANSITION STRUCTURE	T.S.
MANHOLE	M.H.
LOCAL DEPRESSION	L.D.
REINFORCED CONCRETE PIPE	R.C.P.
TEST BORING	T.B.
JUNCTION STRUCTURE	J.S.
CATCH BASIN	C.B.
SANITARY SEWER	S.S.
TEMPORARY BENCH MARKS	T.B.M.
SOUTHERN CALIFORNIA GAS CO.	SCG
L.A. DEPT. OF WATER & POWER WATER SYST.	D.W.&P.W.S.
SOUTHERN CALIFORNIA WATER CO.	SCW
INGLEWOOD CITY WATER DEPT.	I.W.D.
SOUTHERN CALIFORNIA EDISON CO.	S.C.E.
PACIFIC TELEPHONE & TELEGRAPH CO.	P.T.&T.
RICHFIELD OIL CO. GAS LINE	R.G.L.
HOLLYWOOD TURF CLUB WATER	H.T.C.W.
UNIFIED SCHOOL DISTRICT WATER	U.S.D.W.
BURRIED CABLE	BUR. CA.
CRUSHED AGGREGATE BASE	C.A.B.
LIGHT STANDARD	L.S.

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
STANDARD DRAWINGS

DRAWING NO.	TITLE
2-D 88	LOCAL DEPRESSION NO.2
2-D 96	STANDARD DROP STEP
2-D104	MANHOLE NO.3
2-D107	CONCRETE RINGS, REDUCER AND PIPE FOR MANHOLE SHAFT
2-D156	MANHOLE FRAME AND COVER FOR CATCH BASINS
2-D157	CATCH BASIN REINFORCEMENT FOR ROUND MANHOLES
2-D160	CATCH BASIN NO. 1
2-D162	CATCH BASIN NO. 2
2-D171	STANDARD A-615 REINFORCING BARS
2-D172	CATCH BASIN REINFORCEMENT
2-D173.1 TO .3	PIPE SUPPORTS ACROSS TRENCHES
2-D175	REMOVABLE PROTECTION BAR FOR CATCH BASINS
2-D177	PIPE BEDDING IN TRENCHES
2-D184	MANHOLE NO.2
2-D189	JUNCTION STRUCTURE NO.1
2-D191	JUNCTION STRUCTURE NO.3
2-D193	JUNCTION STRUCTURE NO.4
2-D224	CONNECTION TO CATCH BASIN FOR PIPES 12" THROUGH 72"
2-D232	DETAIL OF CATCH BASIN OPENING
2-D235	TRANSITION STRUCTURE NO.1
2-D239	TRANSITION STRUCTURE NO.2
2-D250	REMODELING OF SANITARY SEWER HOUSE CONNECTIONS
2-D251	PROTECTION FOR MAIN LINE AND HOUSE CONNECTION SEWERS
2-D264	ADJUSTABLE PROTECTION BAR STIRRUP
2-D393	CONCRETE COLLAR FOR PIPES 12" THROUGH 66"
2-D466	CRITERIA FOR THE DESIGN OF SHORING FOR EXCAVATIONS
2-D400	SAMPLE SHEET FOR USE AS A GUIDE IN PREPARING CALCULATIONS FOR SHORING OF EXCAVATIONS
2-D413	UNIFIED SOIL CLASSIFICATION SYSTEM
2-D465	ADDITIONAL REINFORCEMENT FOR JACKED R.C. BOX
2-D431	"D" LOAD TABLE FOR DESIGN OF ASBESTOS CEMENT PIPE
2-D461	MODIFICATIONS FOR SIDE OPENING CATCH BASINS
2-D472	STANDARD 24 INCH MANHOLE FRAME AND COVER
2-D476	PORTABLE SECURITY FENCE FOR OPEN TRENCHES

LOS ANGELES COUNTY ENGINEER
STANDARD DRAWINGS

DRAWING NO.	TITLE
S-3	BRICK MANHOLE
S-17	STANDARD MANHOLE STEP
S-21	BEDDING FOR SEWER PIPE
S-23	CRADLING AND ENCASEMENT
S-27	CHIMNEY PIPE AND BASE
S-33	ALLOWABLE TRENCH WIDTH
S-35	LOCKING MANHOLE FRAME AND COVER
S-36	NON-REINFORCED PRECAST CONCRETE MANHOLE
LOS ANGELES COUNTY ROAD DEPARTMENT STANDARD DRAWINGS	
M57-39R	PARTIAL CONCRETE REPLACEMENT FOR CROSS GUTTER AND SPANDRELS
32-01	TYPICAL SIDEWALK DETAILS
LOS ANGELES COUNTY SANITATION DISTRICT STANDARD DRAWINGS	
CITY OF INGLEWOOD STANDARD DRAWINGS	
DRAWING NO.	TITLE
S-a-201	STANDARD MANHOLE, TYPE "A"
S-a-204	STANDARD MANHOLE, TYPE "D"
S-a-207	STANDARD 24" MANHOLE FRAME & COVER
S-a-209	STANDARD MANHOLE STEP
S-a-217	CONCRETE PIPE SUPPORT
S-a-220	STANDARD PULL RING
S-a-222	TEMPORARY SEWER SUPPORT
S-a-227	STANDARD CONCRETE BEAM FOR HOUSE CONNECTIONS
G-1	STANDARD CURB & GUTTER AND STANDARD CROSS GUTTER
G-2	STANDARD ALLEY INTERSECTION
G-4	STANDARD DRIVEWAY APPROACH

STRUCTURAL NOTES CONT'D.

- UNLESS OTHERWISE SHOWN ON THE DETAILS, IN CURVED SECTIONS TRANSVERSE BARS SHALL BE PLACED RADIALLY. STRAIGHT TRANSVERSE BARS IN TOP AND BOTTOM SLABS SHALL BE SPACED AS SHOWN ON THE TYPICAL SECTIONS; SPACING SHALL BE AT THE CENTERLINE OF CONSTRUCTION. STRAIGHT BARS AND L-BARS IN WALLS SHALL BE SPACED AS SHOWN ON THE TYPICAL SECTIONS, WITH THE SPACING MEASURED BETWEEN THE VERTICAL LEGS OF BARS.
- AT THE BEGINNING AND ENDING OF ALL POURS, A CURTAIN OF REINFORCEMENT COMPOSED OF B, C, C₂, D, F, G, AND H BARS SHALL BE PLACED THREE INCHES FROM THE TRANSVERSE CONSTRUCTION JOINT.
- IN ALL SECTIONS LAP C AND C₂ BARS, THE VERTICAL LENGTH OF C AND C₂ BARS HAS BEEN CALCULATED FOR A FOUR-INCH STARTER WALL. IF THE HEIGHT OF STARTER IS VARIED, THE VERTICAL LENGTH OF THE C AND C₂ BARS SHALL BE VARIED CORRESPONDINGLY SO AS TO MAINTAIN A 30 DIAMETER LAP BETWEEN THE TWO BARS. THE LAPS SHALL BE BASED ON THE SMALLER BAR.
- IF WALL THICKNESS IS SIX INCHES, PLACE REINFORCEMENT AT THE CENTERLINE OF THE WALL.
- CONCRETE QUANTITIES ARE BASED ON A SIX BY SIX INCH FILLET AND STEEL QUANTITIES DO NOT INCLUDE ANY OPTIONAL SPLICES.
- ALL LONGITUDINAL BARS SHALL BE NO. 4 BARS. SPACING SHALL BE 18 INCHES UNLESS OTHERWISE SHOWN. BARS IN TOP OR BOTTOM SLAB SHALL BE SPACED SYMMETRICAL ABOUT THE CENTER LINE. BARS IN WALLS SHALL BE SPACED SYMMETRICAL ABOUT MID HEIGHT OF THE WALLS.
- CONCRETE BREAKOUT NOTES:
 - FOR CONCRETE REMOVAL, WHERE REINFORCEMENT IS TO BE RETAINED THROUGH THE JOINT, MAKE A SAWCUT ONE AND ONE-HALF INCHES DEEP ON THE EXPOSED FACES OF THE CONCRETE AT THE REMOVAL LIMITS. CUT A GROOVE IN THE CONCRETE ADJACENT TO THE SAWCUT, ON THE SIDE TO BE REMOVED TO THE DEPTH OF THE SAWCUT, WITH A CHIPPING HAMMER. EXISTING REINFORCEMENT SHALL BE RESTORED AND RETAINED OR CUT AND LAPPED 30 BAR DIAMETERS WITH NEW REINFORCEMENT.
 - CARE SHALL BE EXERCISED IN SAWING AT THE REMOVAL LIMITS SO AS NOT TO CUT THE REINFORCEMENT IN THE STRUCTURE TO REMAIN. AFTER MAKING SAW CUTS THE CONCRETE SHALL BE REMOVED WITH HAND OPERATED EQUIPMENT. USE OF EXPLOSIVES, WRECKING BALLS, STOMPERS OR OTHER EQUIPMENT, WHICH WOULD CAUSE DAMAGE TO THE EXISTING FACILITY REMAINING IN PLACE, WILL NOT BE PERMITTED. THE ENGINEER SHALL BE THE SOLE JUDGE OF THE USAGE OF ANY EQUIPMENT FOR THE CONCRETE REMOVAL OPERATION.

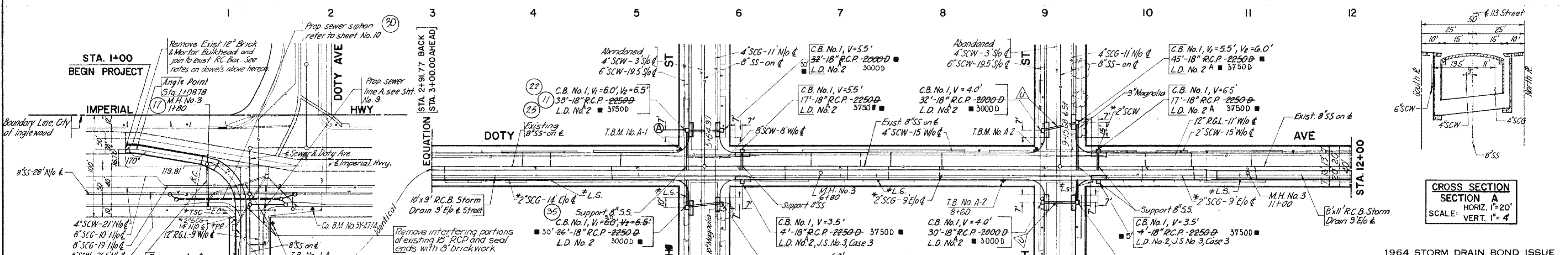
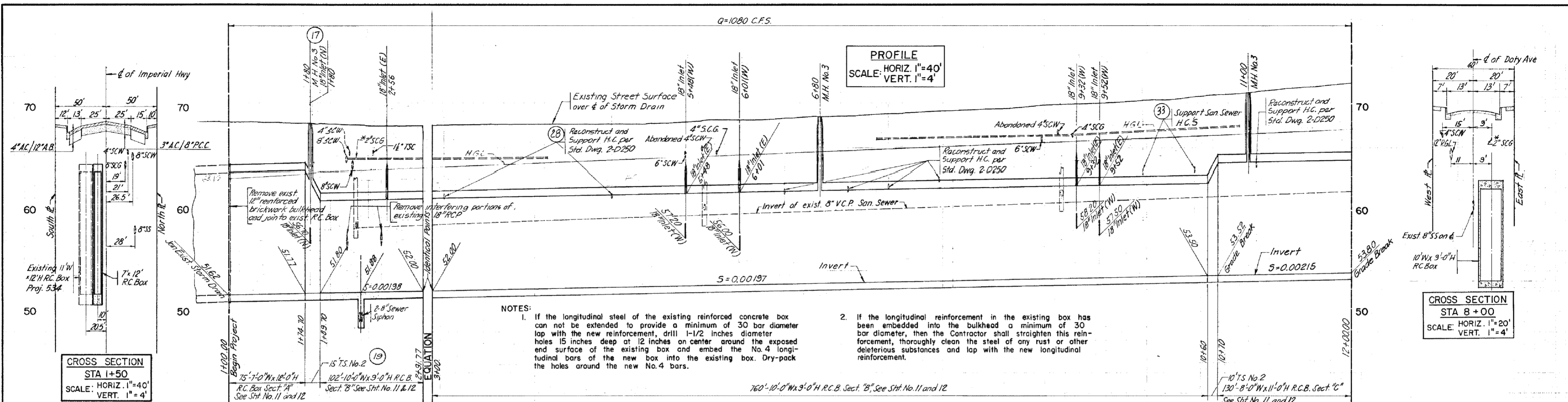


1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: <i>William J. Mohr</i> ASSISTANT CITY ENGINEER R.C.E. 17167		PROJECT NO. 4401 INGLEWOOD LINES A, AND C UNIT I LEGEND AND ABBREVIATION, STANDARD DRAWINGS, AND STRUCTURAL NOTES	
APPROVED BY: <i>John J. ...</i> CITY ENGINEER R.C.E. 11237		RECOMMENDED BY: <i>...</i> DIVISION ENGINEER (DESIGN)	
SCALE AS SHOWN	DATE Sept 74	DWG. NO. 364-4401-D63	SHEET 3 OF 14

REVISIONS			
MARK	DATE	DESCRIPTION	

"AS BUILT" DRAWING



CURVE DATA
 $\Delta = 80^{\circ}01'14''$ BC = 1+74.70
 R = 45.00 EC = 2+31.54
 L = 62.84
 T = 37.77

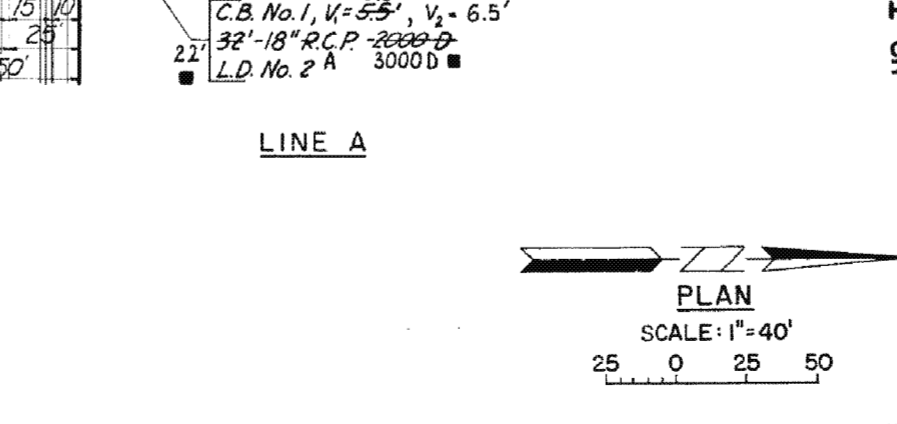
REVISIONS			
MARK	DATE	DESCRIPTION	
1	10-2-80	As Built	

PLAN
 SCALE: 1" = 40'
 25 0 25 50

Co.B.M. No. 54-4774; L & Spk. in N curb of Imperial Hwy. 36'± W/O B.C.R. N.W. cor. Doty Ave. and Imperial Hwy. EL. 67.773

T.B.M. No. A-1; L & Pk. in curb return S/W cor. of Doty Ave. and 113 Th St. 7' N/O B.C.R. EL. 68.878

T.B.M. No. A-2; L & Pk. in curb return S/W cor. of Doty Ave. and 112 Th St. 7' N/O B.C.R. EL. 70.420



1964 STORM DRAIN BOND ISSUE

LOS ANGELES COUNTY
 FLOOD CONTROL DISTRICT

CITY OF INGLEWOOD
 PUBLIC WORKS DEPARTMENT
 ENGINEERING DIVISION

PROJECT NO. 4401
 INGLEWOOD
 LINE A UNIT I
 CONCRETE CONDUIT

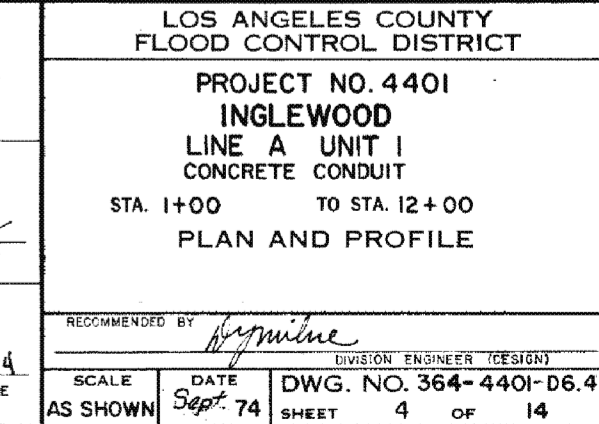
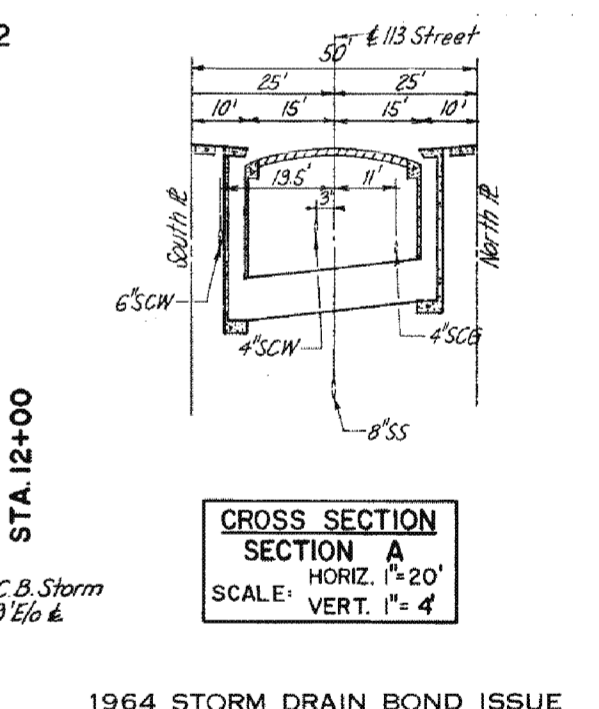
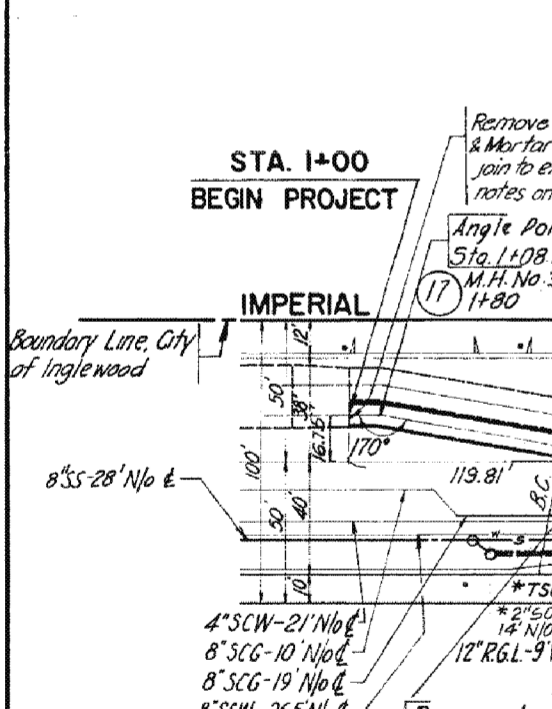
STA. 1+00 TO STA. 12+00
 PLAN AND PROFILE

SUBMITTED BY:
William A. Mason
 ASSISTANT CITY ENGINEER R.C.E. 17167

APPROVED BY:
William A. Mason 1-31-74
 CITY ENGINEER R.C.E. 1237

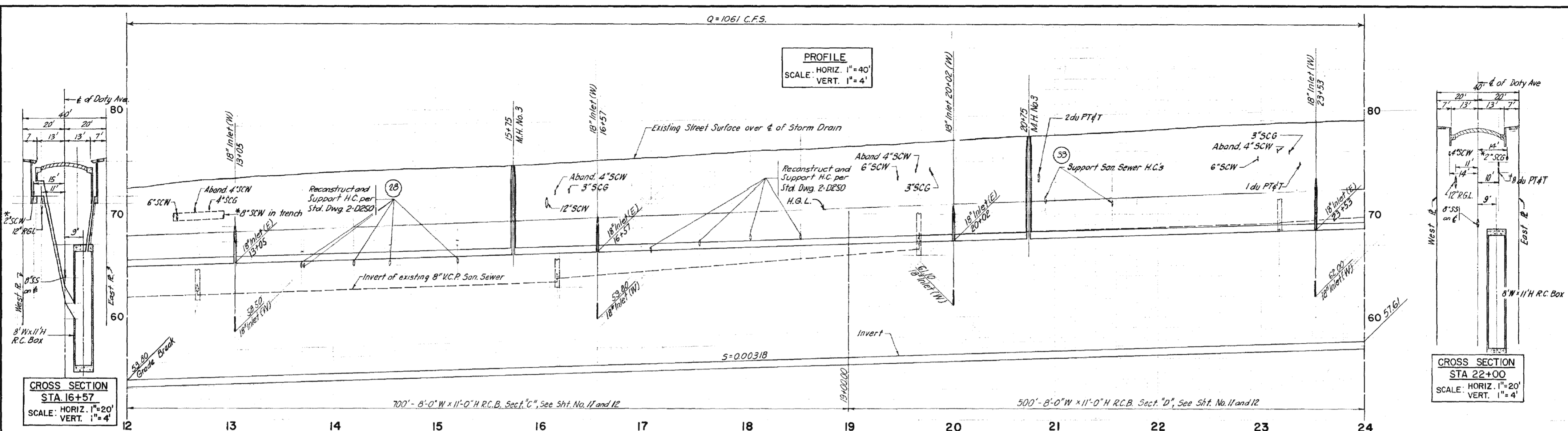
RECOMMENDED BY:
Armine
 DIVISION ENGINEER (DESIGN)

SCALE AS SHOWN DATE DWG. NO. 364-4401-D6.4 SHEET 4 OF 14



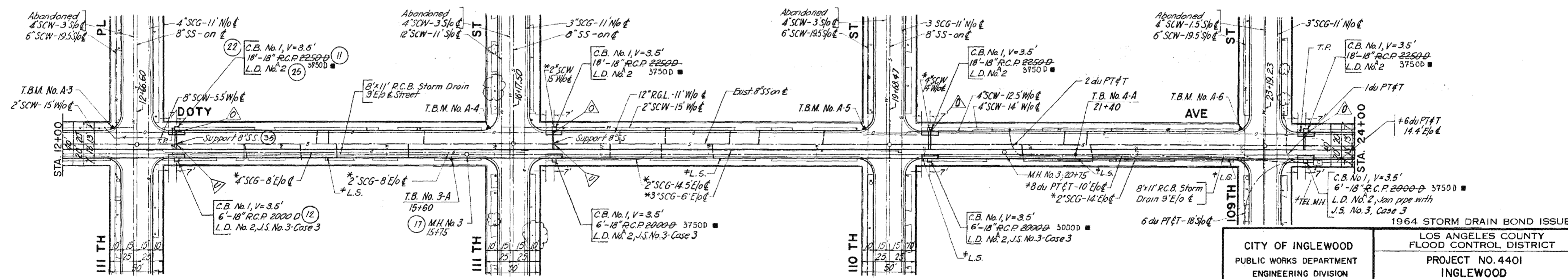
Q=1061 C.F.S.

PROFILE
HORIZ. 1"=40'
VERT. 1"=4'



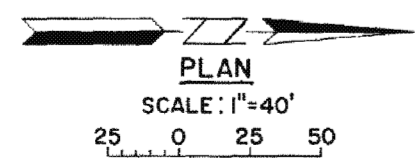
CROSS SECTION
STA. 16+57
SCALE: HORIZ. 1"=20'
VERT. 1"=4'

CROSS SECTION
STA. 22+00
SCALE: HORIZ. 1"=20'
VERT. 1"=4'



LINE A

REVISIONS	
MARK	DESCRIPTION
1	As Built



T.B.M. No. A-3 L&Pk. in curb return SW cor. of Duty Ave. and 111 th Pl. 7' N/o B.C.R. El. 70.420

T.B.M. No. A-4 L&Pk. in curb return SW cor. of Duty Ave. and 111 th St. 7' N/o B.C.R. El. 74.868

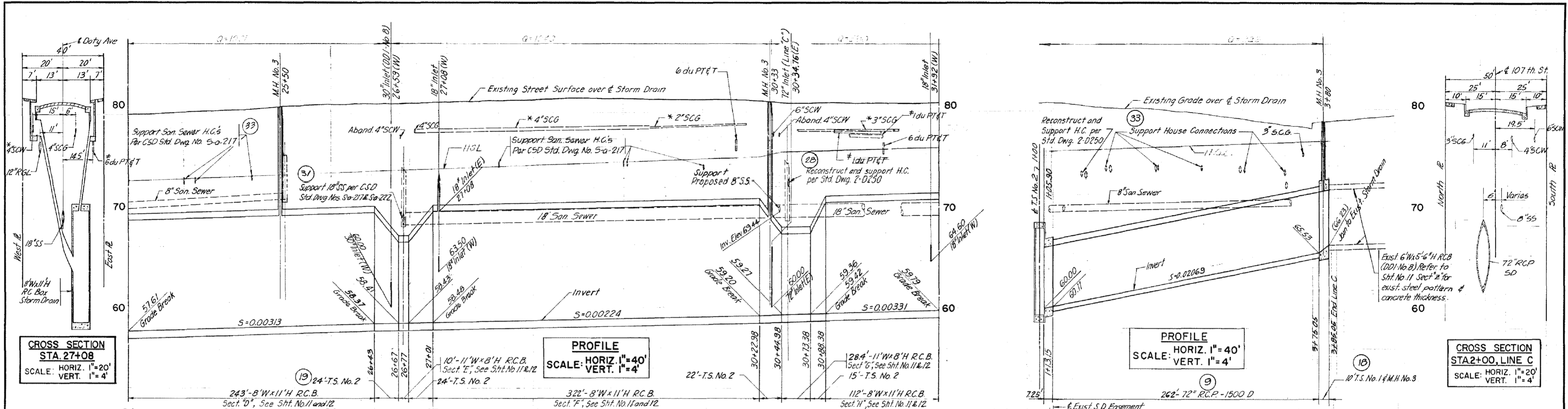
T.B.M. No. A-5 L&Pk. in curb return SW cor. of Duty Ave. and 110 th St. 7' N/o B.C.R. El. 76.889

T.B.M. No. A-6 L&Pk. in curb return SW cor. of Duty Ave. and 109 th St. 7' N/o B.C.R. El. 78.896



CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: <i>William Maher</i> ASSISTANT CITY ENGINEER R.C.E. 17167		PROJECT NO. 4401 INGLEWOOD LINE A UNIT I CONCRETE CONDUIT STA. 12+00 TO STA. 24+00 PLAN AND PROFILE	
APPROVED BY: <i>William Maher</i> CITY ENGINEER R.C.E. 11237		RECOMMENDED BY: <i>Rymilue</i> DIVISION ENGINEER (DESIGN)	
SCALE AS SHOWN	DATE Sept 74	DWG. NO. 364-4401-06.5 SHEET 5 OF 14	

"AS SHOWN" DRAWING

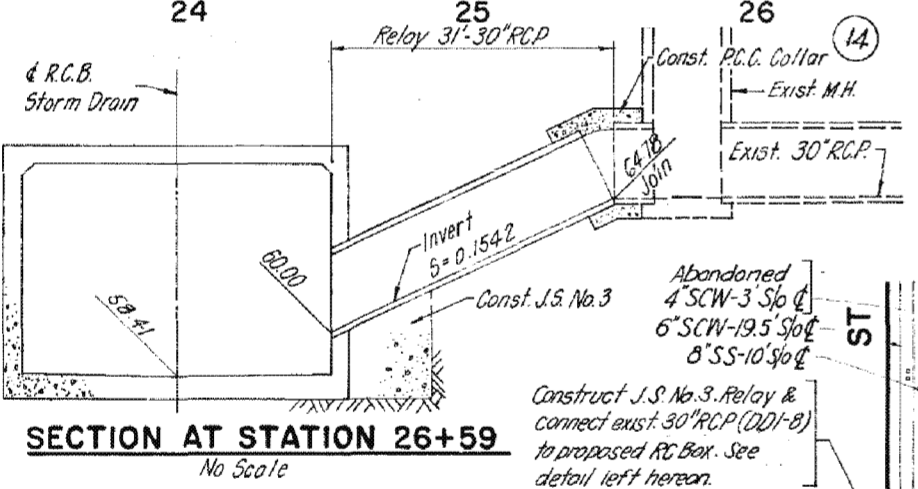


**CROSS SECTION
STA. 27+08**
SCALE: HORIZ. 1"=20'
VERT. 1"=4'

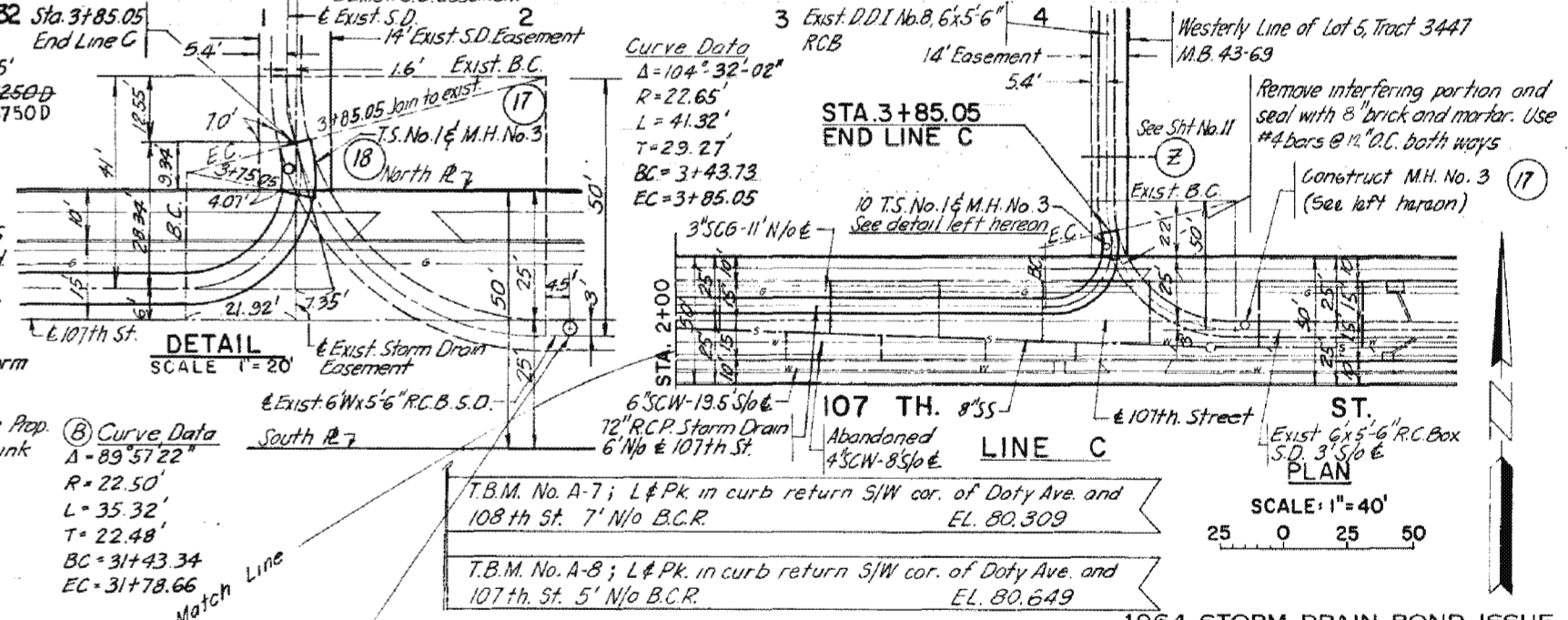
**PROFILE
SCALE: HORIZ. 1"=40'
VERT. 1"=4'**

**PROFILE
SCALE: HORIZ. 1"=40'
VERT. 1"=4'**

**CROSS SECTION
STA. 2+00, LINE C**
SCALE: HORIZ. 1"=20'
VERT. 1"=4'



SECTION AT STATION 26+59
No Scale



**DETAIL
SCALE 1"=20'**

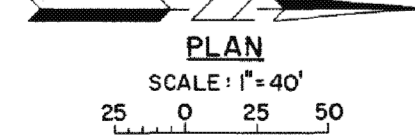
Curve Data
 $\Delta = 104^\circ 32' 08''$
 $R = 22.65'$
 $L = 41.32'$
 $T = 29.27'$
 $BC = 3+43.73$
 $EC = 3+85.05$

Curve Data
 $\Delta = 89^\circ 57' 22''$
 $R = 22.50'$
 $L = 35.32'$
 $T = 22.48'$
 $BC = 3+43.34$
 $EC = 3+78.66$

A Curve Data:
 $\Delta = 89^\circ 58' 46''$
 $R = 22.50'$
 $L = 35.33'$
 $T = 22.49'$
 $BC = 30+37.76$
 $EC = 30+73.09$

C Curve Data
 $\Delta = 45^\circ 01' 14''$
 $R = 45.0'$
 $L = 35.36'$
 $T = 18.65'$
 $BC = 1+13.15$
 $EC = 1+48.51$

REVISIONS		
MARK	DATE	DESCRIPTION
■	10-2-00	As Built

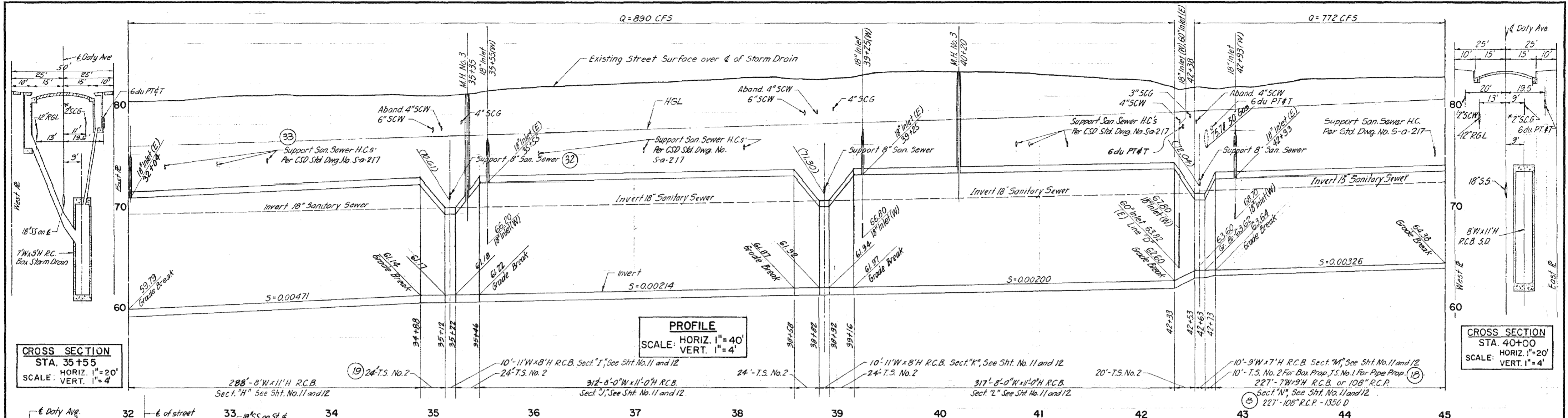


**PLAN
SCALE: 1"=40'**



CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION SUBMITTED BY: <i>William Mahar</i> ASSISTANT CITY ENGINEER R.C.E. 17167 APPROVED BY: <i>William Mahar</i> 1-31-74 CITY ENGINEER R.C.E. 1237	PROJECT NO. 4401 INGLEWOOD LINES A & C UNIT I CONCRETE CONDUIT STA. 24+00 TO STA. 32+00 LINE A STA. 1+00 TO STA. 3+85.05 LINE C PLAN AND PROFILE	
	RECOMMENDED BY: <i>William Mahar</i> DIVISION ENGINEER (DESIGN)	SCALE: AS SHOWN DATE: Sept. 74 DWG. NO. 364-4401-D6.6 SHEET 6 OF 14
	1964 STORM DRAIN BOND ISSUE LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	

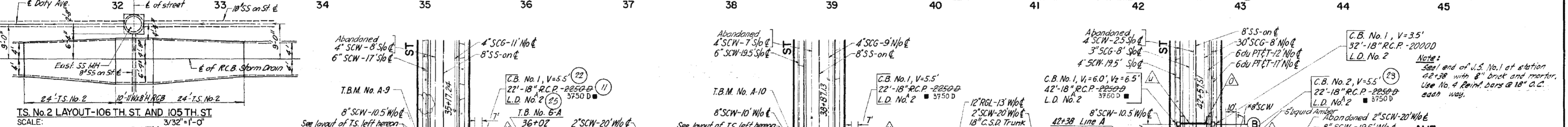
"AS BUILT" DRAWING



CROSS SECTION
 STA. 35+55
 HORIZ. 1"=20'
 SCALE: VERT. 1"=4'

PROFILE
 SCALE: HORIZ. 1"=40'
 VERT. 1"=4'

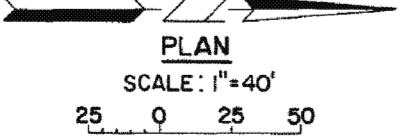
CROSS SECTION
 STA. 40+00
 HORIZ. 1"=20'
 SCALE: VERT. 1"=4'



T.S. No. 2 LAYOUT-106 TH. ST. AND 105 TH. ST.
 SCALE: 3/32"=1'-0"

CROSS SECTION
 SECTION B
 HORIZ. 1"=20'
 SCALE: VERT. 1"=4'

REVISIONS	
MARK	DATE
■	10-2-80
	As Built



T.B.M. No. A-9; L&Pk. in curb return S/W cor. Doty Ave. and 106 th St. 6' N/O B.C.R. El. 80.967
 T.B.M. No. A-10; L&Pk. in curb return S/W cor. Doty Ave. and 105 th St. 6' N/O B.C.R. El. 82.681
 T.B.M. No. A-11; L&Pk. in curb return S/W cor. Doty Ave. and 104 th St. El. 81.825

1964 STORM DRAIN BOND ISSUE

LOS ANGELES COUNTY
 FLOOD CONTROL DISTRICT

CITY OF INGLEWOOD
 PUBLIC WORKS DEPARTMENT
 ENGINEERING DIVISION

PROJECT NO. 4401
 INGLEWOOD
 LINE A UNIT I
 CONCRETE CONDUIT
 STA. 32+00 TO STA. 45+00
 PLAN AND PROFILE

SUBMITTED BY:
William Mahan
 ASSISTANT CITY ENGINEER RCE 17167

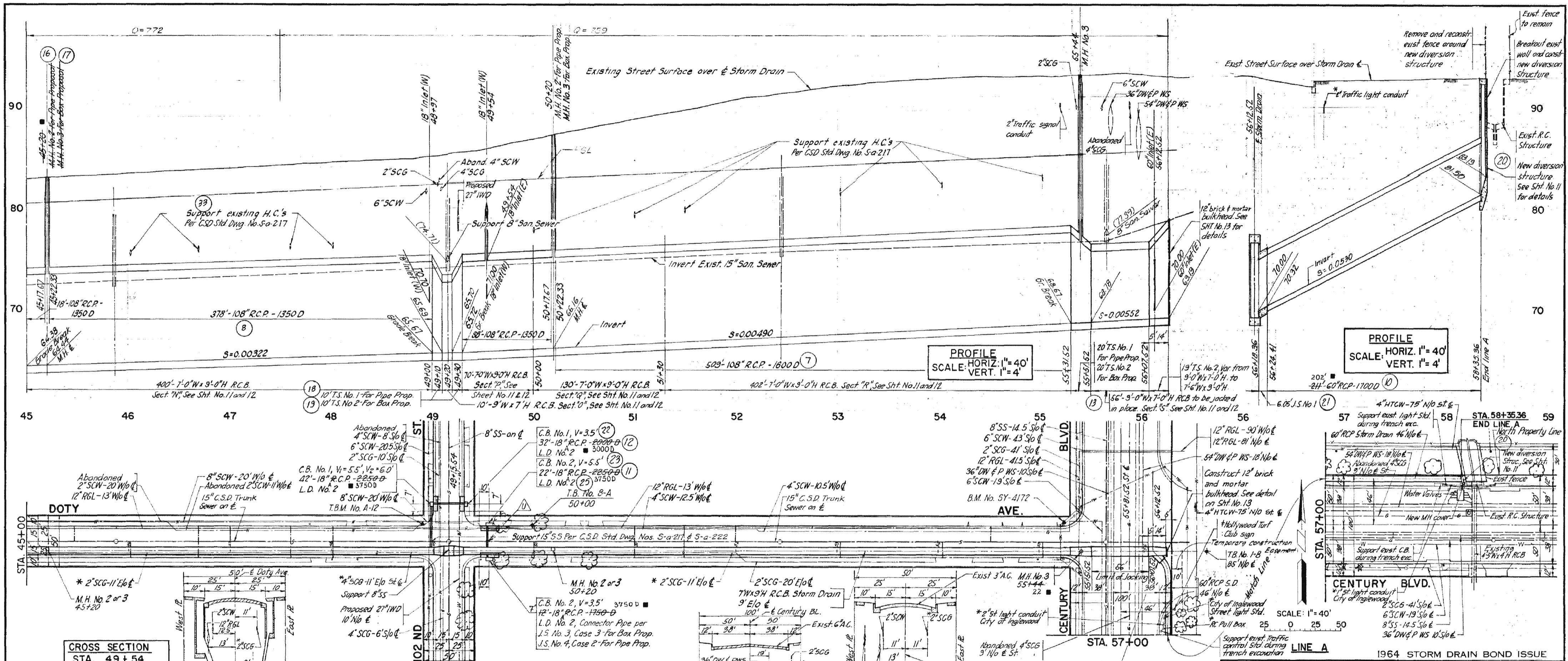
APPROVED BY:
W. J. ...
 CITY ENGINEER RCE 11237

RECOMMENDED BY:
R. ...
 DIVISION ENGINEER (DESIGN)

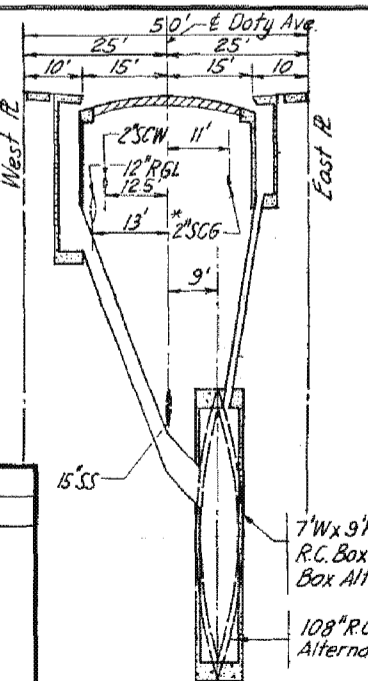
SCALE: AS SHOWN
 DATE: Sept. 74
 DWG. NO. 364-4401-D6.7
 SHEET 7 OF 14



"AS BUILT" DRAWING

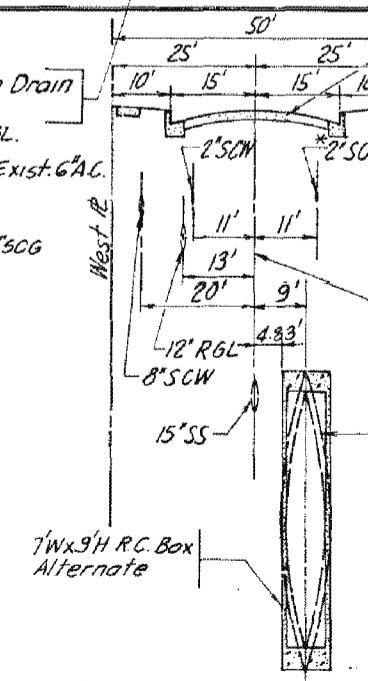


CROSS SECTION
STA. 49+54
SCALE: HORIZ. 1"=20'
VERT. 1"=4'

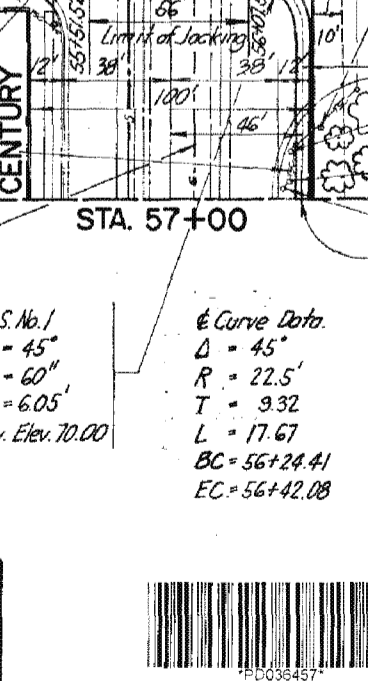


PLAN
SCALE 1"=40'

CROSS SECTION
STA. 57+00 LINE A
SCALE: HORIZ. 1"=40'
VERT. 1"=8'



CROSS SECTION
STA. 47+00
SCALE: HORIZ. 1"=20'
VERT. 1"=4'



REVISIONS			
MARK	DATE	DESCRIPTION	
1	10-2-80	As Built	

T.B.M. No. A12 - L&PK in curb return on SW cor. of Doty Ave. & 102nd St. 5' N/O B.C.R. EL. 85.061
B.M. No. 5Y-4172 - L&T 4' W/O B.C.R. on SW cor. of Doty Ave. & Century Blvd. (L.A. Co. Road Dept.) EL. 93.162

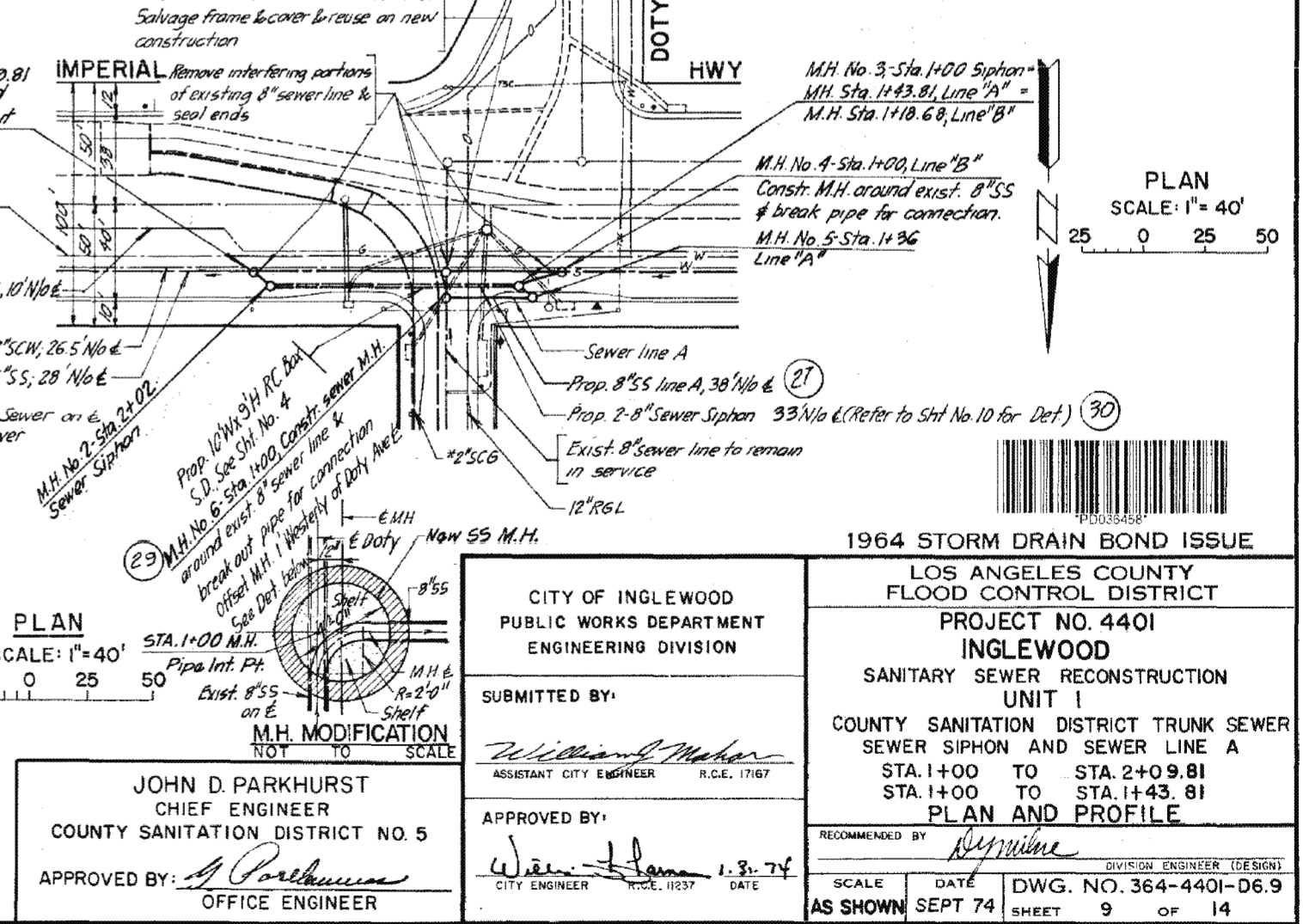
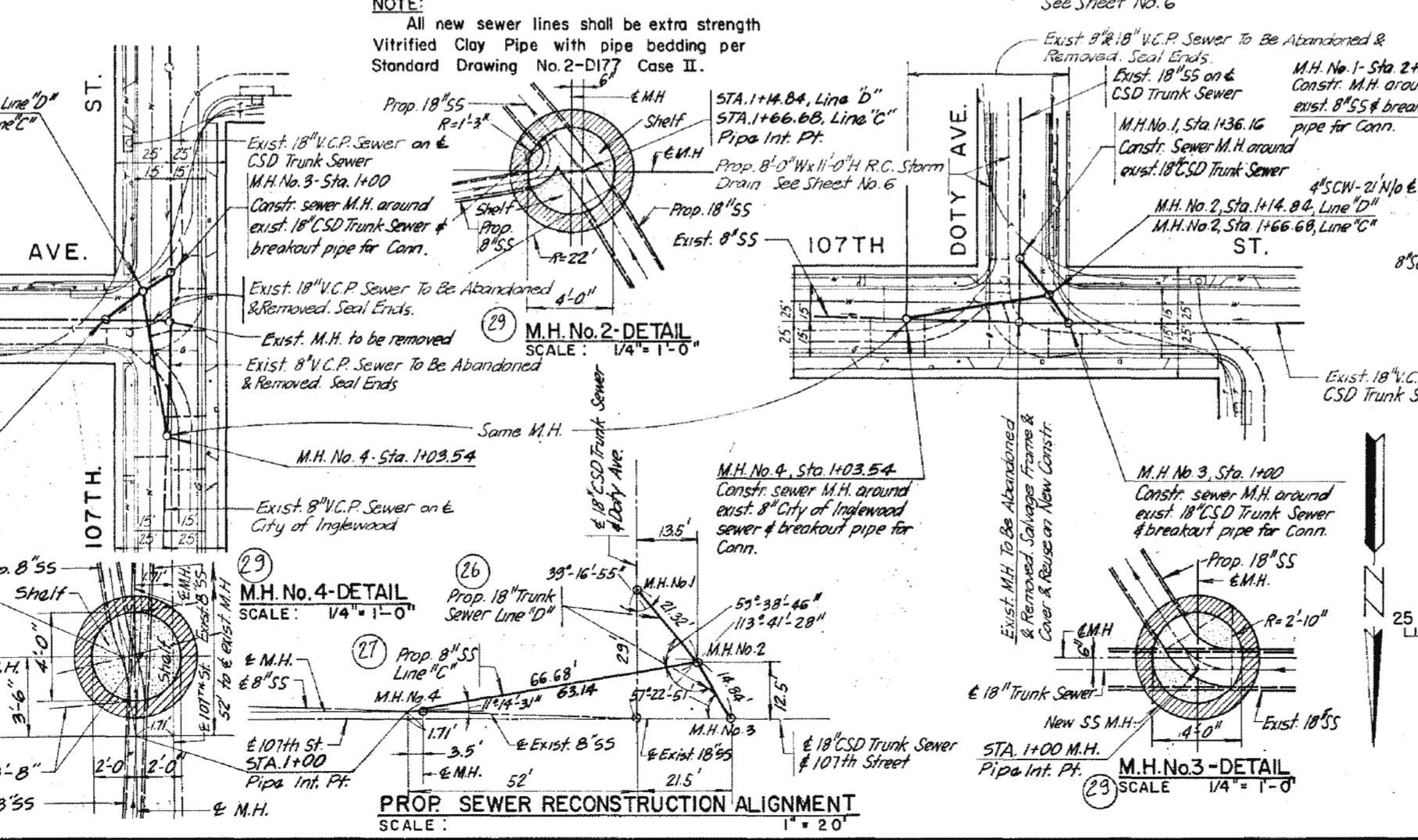
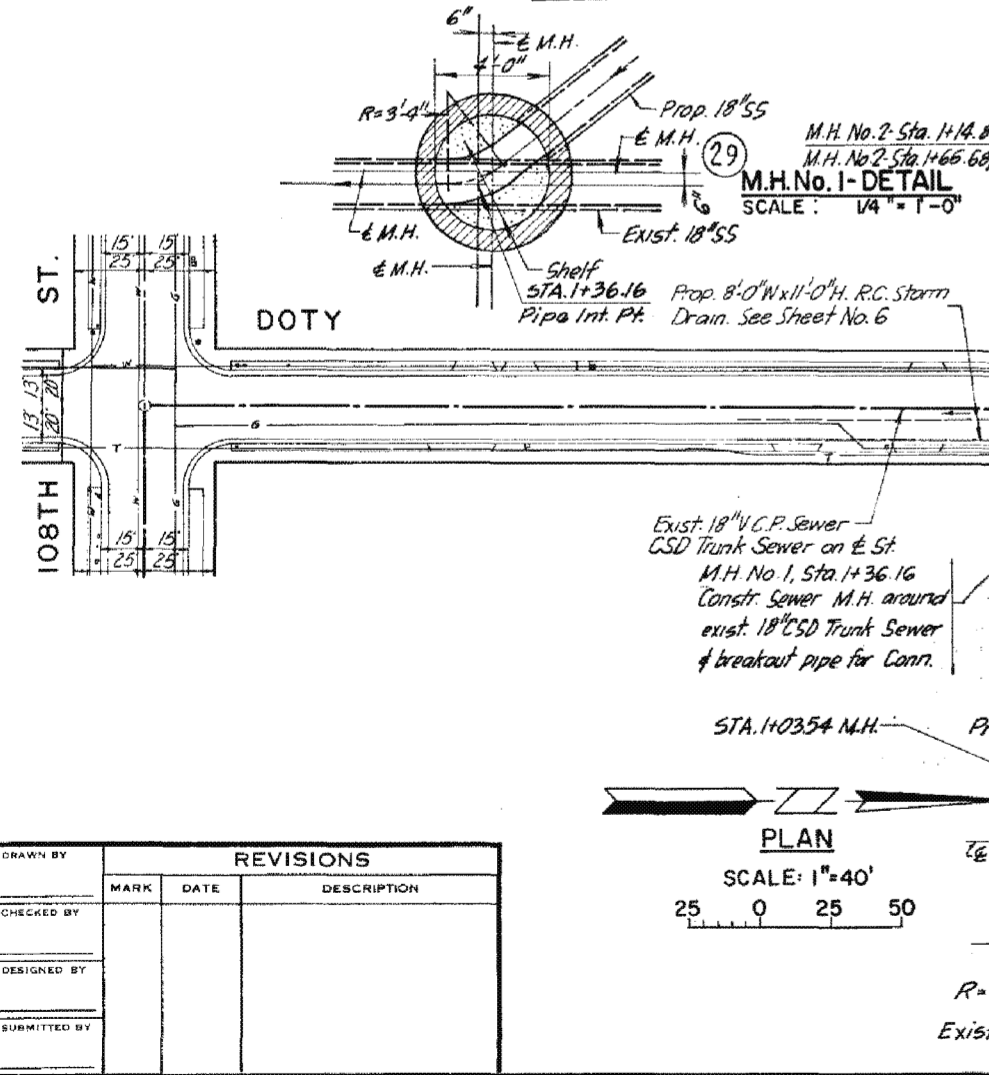
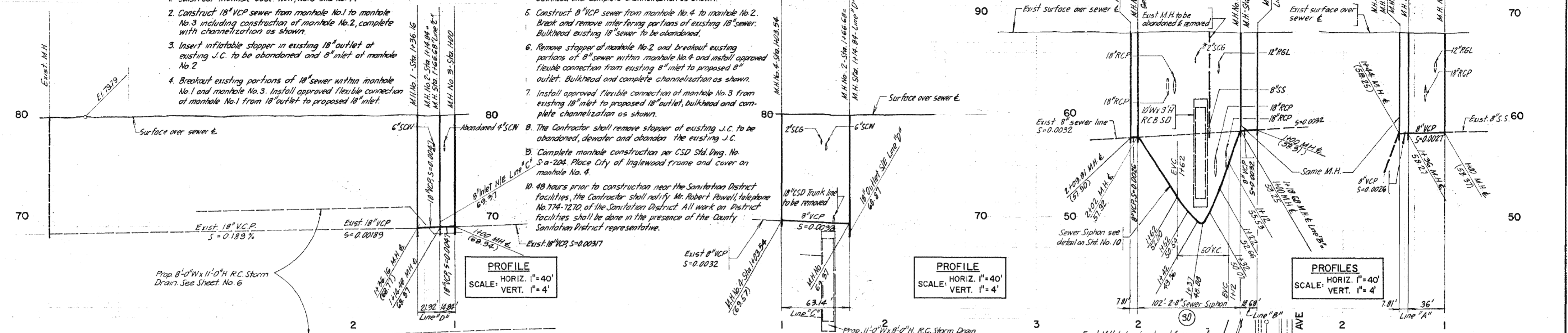
CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION SUBMITTED BY: <i>William Mahan</i> ASSISTANT CITY ENGINEER RCE 17167	1964 STORM DRAIN BOND ISSUE
	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT PROJECT NO. 4401 INGLEWOOD LINE A UNIT I CONCRETE CONDUIT LINE A STA. 45+00 TO STA. 58+35.36 PLAN AND PROFILE
APPROVED BY: <i>William Mahan</i> CITY ENGINEER RCE 11237 DATE: 1.31.74	RECOMMENDED BY: <i>Agmine</i> DIVISION ENGINEER (DESIGN) SCALE: AS SHOWN DATE: 7/22/74 DWG. NO. 364-4401-D6.8 SHEET 8 OF 14

"AS BUILT" DRAWING

**SEQUENCE OF WORK FOR THE RECONSTRUCTION OF EXISTING 18 INCH
SANITATION DISTRICT SEWER AT DOTY AVENUE & 107TH ST.**

During Low Flow Period and Under the Direction of the Sanitation District Representative,
the Contractor Shall Comply with the Following:

1. Construct manhole base No.1, No.3 and No.4.
2. Construct 18" VCP sewer from manhole No.1 to manhole No.3 including construction of manhole No.2, complete with channelization as shown.
3. Insert inflatable stopper in existing 18" outlet at existing J.C. to be abandoned and 8" inlet at manhole No.2
4. Breakout existing portions of 18" sewer within manhole No.1 and manhole No.3. Install approved flexible connection at manhole No.1 from 18" outlet to proposed 18" inlet.
5. Construct 8" VCP sewer from manhole No.4 to manhole No.2. Break and remove interfering portions of existing 18" sewer. Bulkhead existing 18" sewer to be abandoned.
6. Remove stopper of manhole No.2 and breakout existing portions of 8" sewer within manhole No.4 and install approved flexible connection from existing 8" inlet to proposed 8" outlet. Bulkhead and complete channelization as shown.
7. Install approved flexible connection of manhole No.3 from existing 18" inlet to proposed 18" outlet, bulkhead and complete channelization as shown.
8. The Contractor shall remove stopper of existing J.C. to be abandoned, dewater and abandon the existing J.C.
9. Complete manhole construction per CSD Std. Dwg. No. S-a-204. Place City of Inglewood frame and cover on manhole No.4.
10. 48 hours prior to construction near the Sanitation District facilities, the Contractor shall notify Mr. Robert Powell, telephone No.774-7270 of the Sanitation District. All work on District facilities shall be done in the presence of the County Sanitation District representative.



REVISIONS			
MARK	DATE	DESCRIPTION	

JOHN D. PARKHURST
CHIEF ENGINEER
COUNTY SANITATION DISTRICT NO. 5

APPROVED BY: *[Signature]*
OFFICE ENGINEER

WILLIAM M. MOHAR
ASSISTANT CITY ENGINEER
R.C.E. 17167

APPROVED BY: *[Signature]*
CITY ENGINEER
DATE: 1.31.74

1964 STORM DRAIN BOND ISSUE

LOS ANGELES COUNTY
FLOOD CONTROL DISTRICT

PROJECT NO. 4401

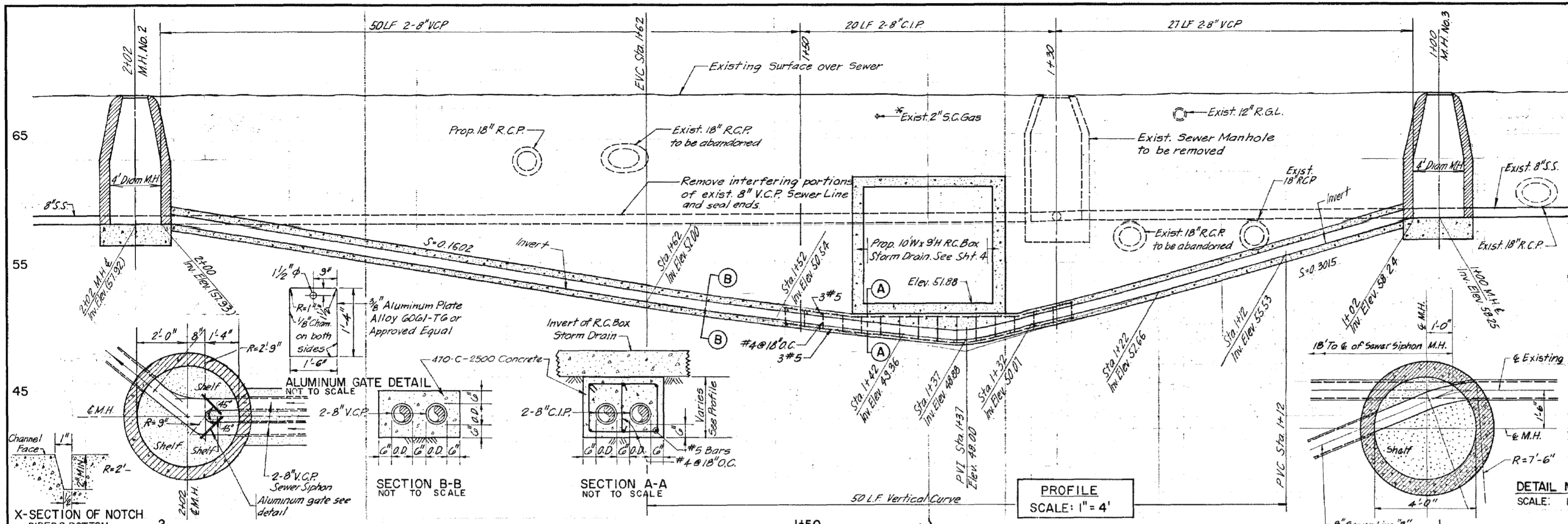
INGLEWOOD

SANITARY SEWER RECONSTRUCTION
UNIT 1

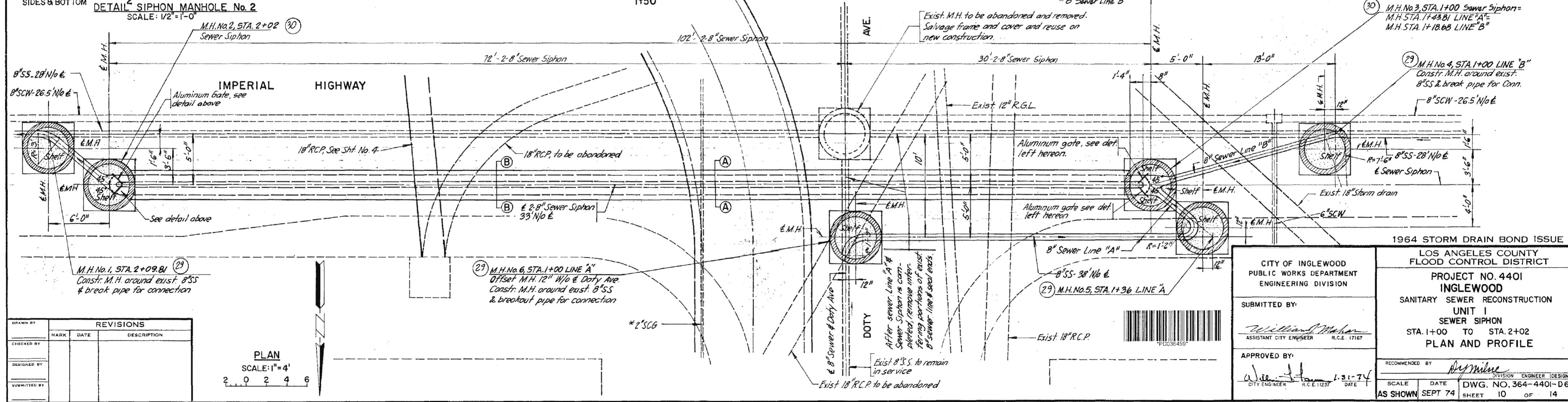
COUNTY SANITATION DISTRICT TRUNK SEWER
SEWER SIPHON AND SEWER LINE A
STA. 1+00 TO STA. 2+09.81
STA. 1+00 TO STA. 1+43.81

PLAN AND PROFILE

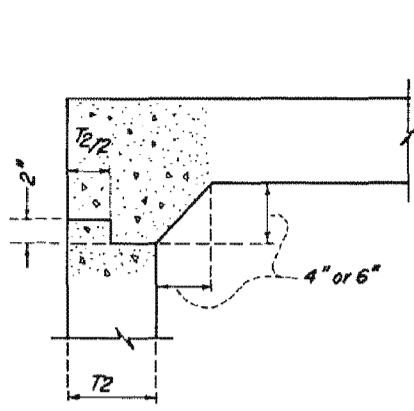
SCALE	DATE	DWG. NO.	DESIGNER
AS SHOWN	SEPT 74	364-4401-06.9	9 OF 14



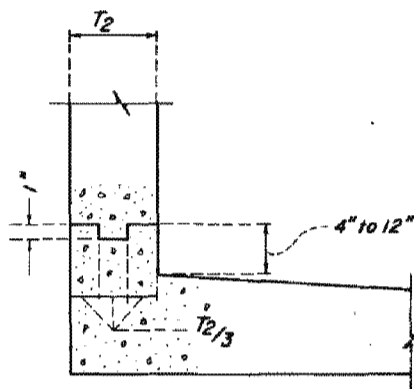
- GENERAL NOTES:** (Applicable to City of Inglewood Sewer Reconstruction and Sewer Siphon Sht. 9 and 10)
1. Use Std. City of Inglewood Dwg. for reconstruction of City of Inglewood sewer lines.
 2. Use extra strength vitri. fired clay pipes, unglazed, bell and spigot, conforming to ASTM C700. Joints shall be type "F" or "G" per standard specifications for C.I.P. use Class 150 per Std. Specifications.
 3. All structures shall be either brick manhole or precast concrete manhole per Std. Dwg.
 4. Use Std. M.H. Frame and Cover per Std. Dwg. S-35
 5. Abandoned sewers shall be sealed with brick and cement mortar.
 6. Resurface trenches within the paved area of street as noted on the storm drain plans.
 7. Refer to Std. Specifications and Special Provisions for sewer testing for sewer leakage.
 8. All joints between cast iron pipe and vitrified clay pipe shall be made with rubber sleeve joint, type "C" or "D" (with bushing if necessary) per standard specifications.



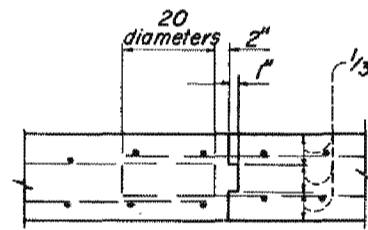
"AS BUILT" DRAWING



TOP OF WALL



BASE OF WALL



TRANSVERSE JOINT

CONSTRUCTION JOINT DETAILS
NO SCALE

DESIGN DATA:

LIVE LOAD:

H20-S16-44 unless otherwise noted

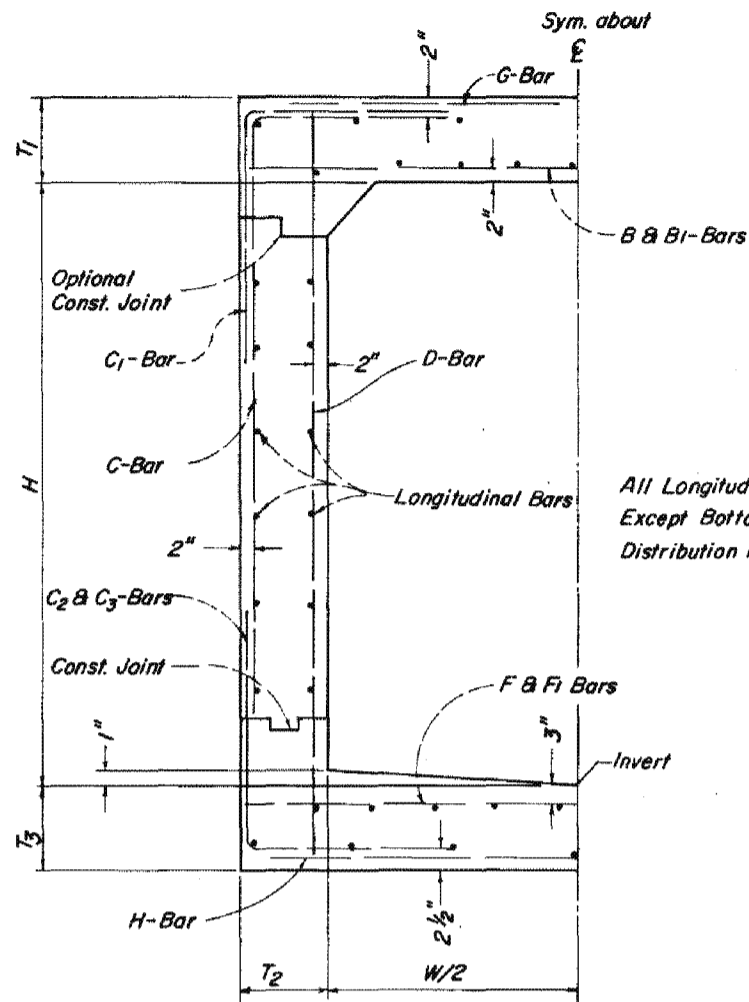
DEAD LOAD:

Earth load per Marston's formula: $W=120$ PCF
 $K_u = K_u' = 0.150$
 B_d = Outside width of box plus 3 feet
 Side earth $W = 40$ PSF per foot of depth
 Internal water pressure: 62.4 p.s.f. per foot of depth
 Weight of Concrete: 150 p.c.f.

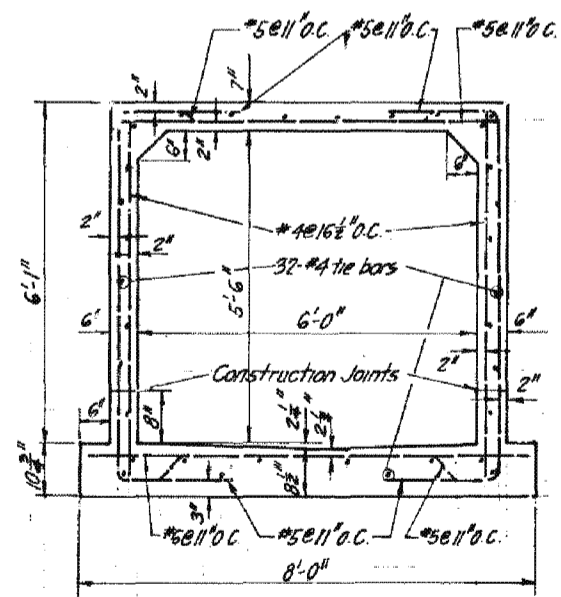
ALLOWABLE STRESSES:

$f_c = 4000$ p.s.i. at 28 days
 $f_s = 1800$ p.s.i.
 $f_s = 24,000$ p.s.i.
 $n = 8$
 shear and bond stresses per A.C.I. 318-63

REVISIONS		
NO.	DATE	DESCRIPTION
1	12-9-74	Changes in Details for Diversion Structure

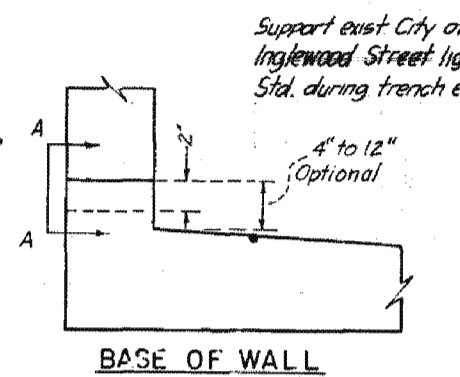
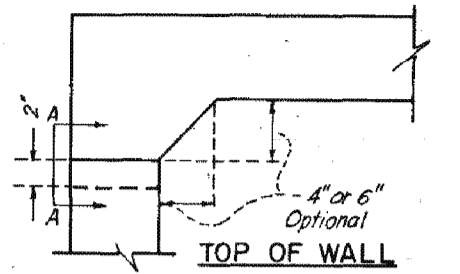


TYPICAL R.C. BOX SECTION
NO SCALE



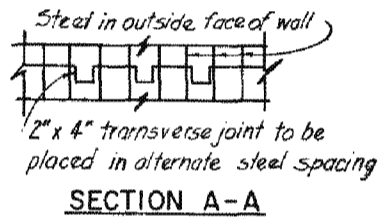
SECTION Z (Refer to Sheet No. 6)
SCALE: 1/2\"/>

CONSTRUCTION JOINT DETAILS
FOR BOXES JACKED IN PLACE

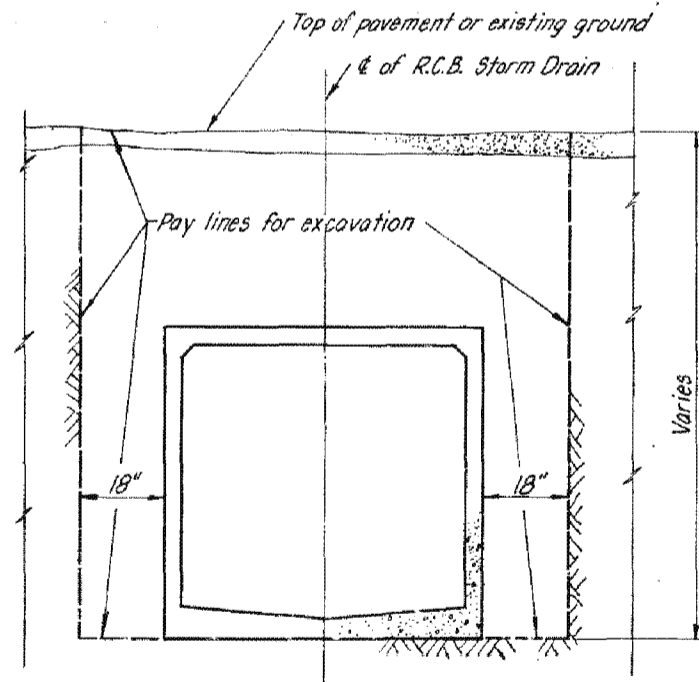


BASE OF WALL

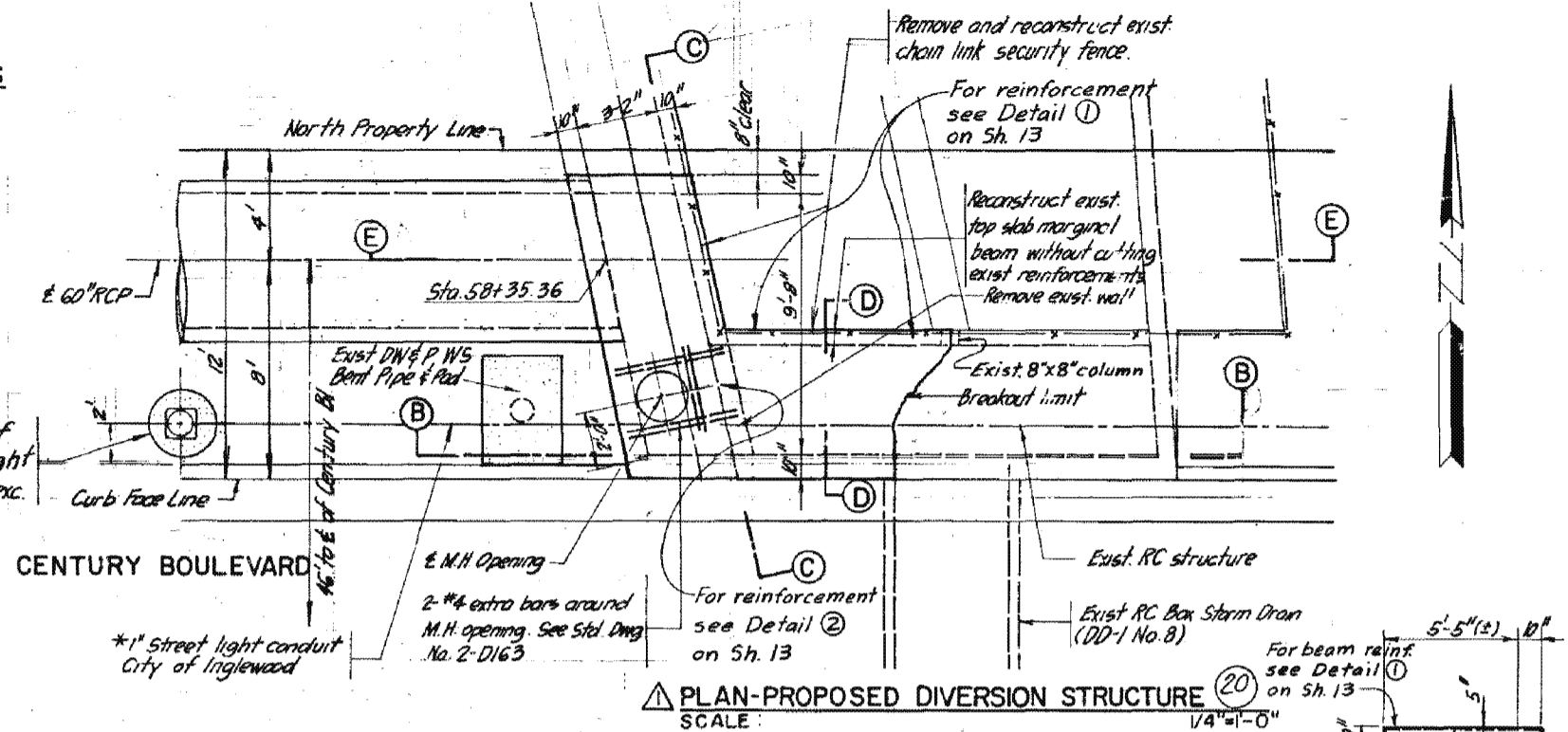
Refer to Std. Dwg. No. 2-D465 for additional steel details



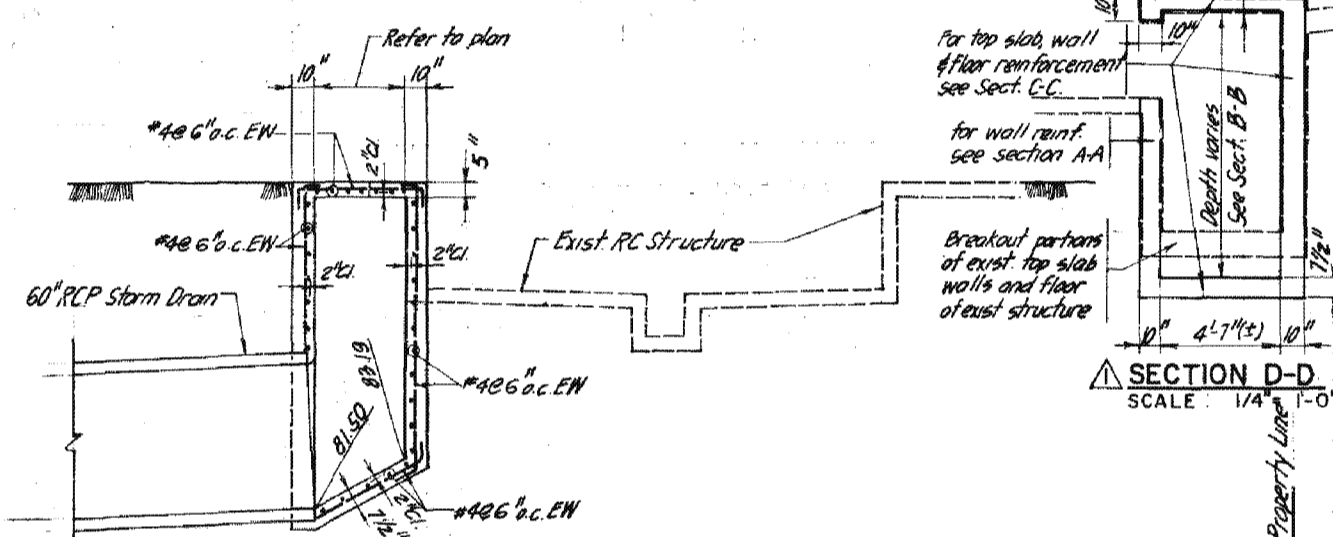
SECTION A-A



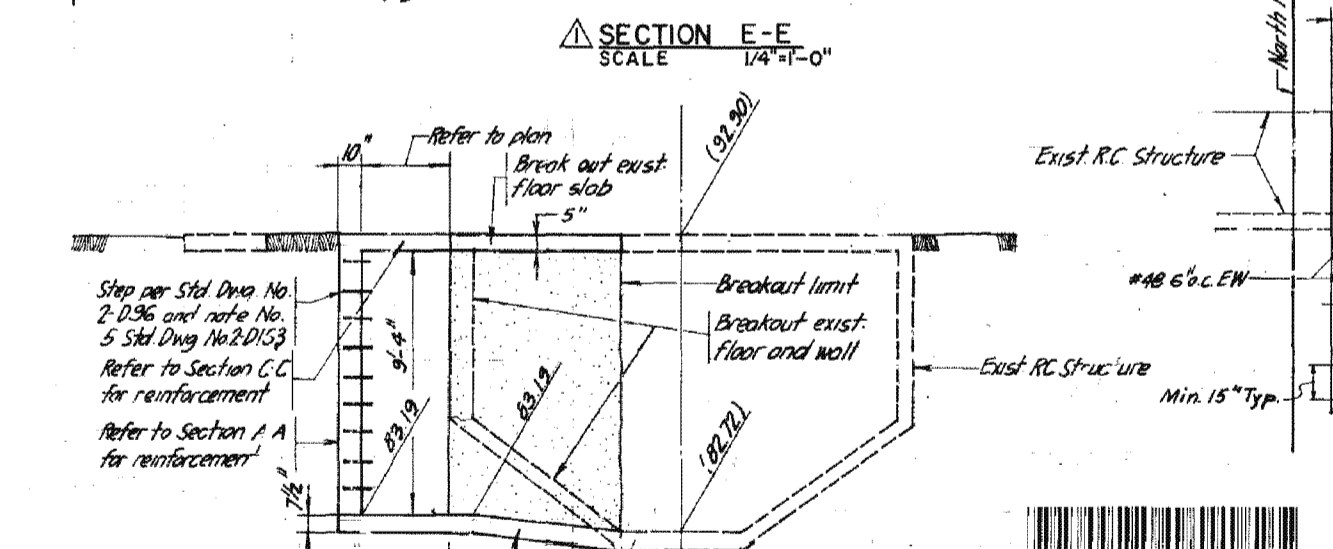
TYPICAL SECTION SHOWING PAY LINES
FOR EXCAVATION OF R.C.B.
NO SCALE



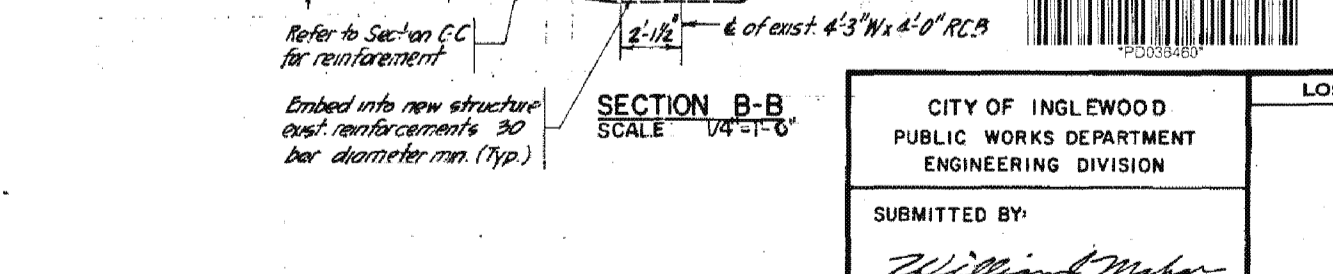
PLAN-PROPOSED DIVERSION STRUCTURE
SCALE: 1/4\"/>



SECTION D-D
SCALE: 1/4\"/>



SECTION E-E
SCALE: 1/4\"/>



SECTION B-B
SCALE: 1/4\"/>

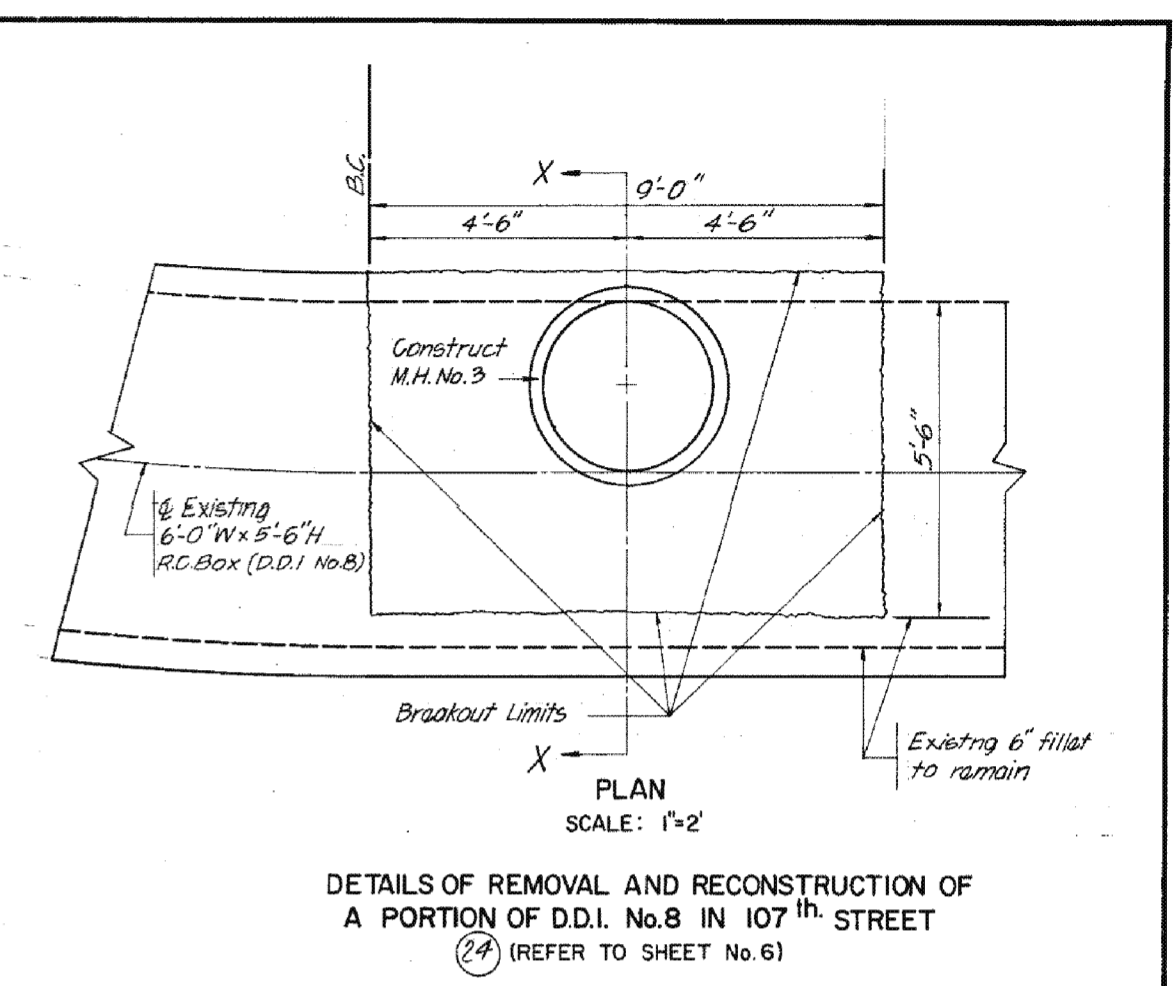
REINFORCED CONCRETE
BOX LOCATION SCHEDULE

BOX SECTION	LINE A STATION	
	FROM	TO
A	1+00	1+74.70
B	1+89.70	2+91.77
C	3+00	10+60
D	10+70	19+00
E	19+00	26+43
F	26+67	26+77
G	30+44.98	30+73.38
H	30+88.38	34+88
I	35+12	35+22
J	35+46	38+58
K	38+82	38+92
L	39+16	42+33
M	42+53	42+63
N	42+73	49+00
O	49+10	49+20
P	49+30	50+00
Q	50+00	51+30
R	51+30	55+31.52
S	55+51.52	56+07.52

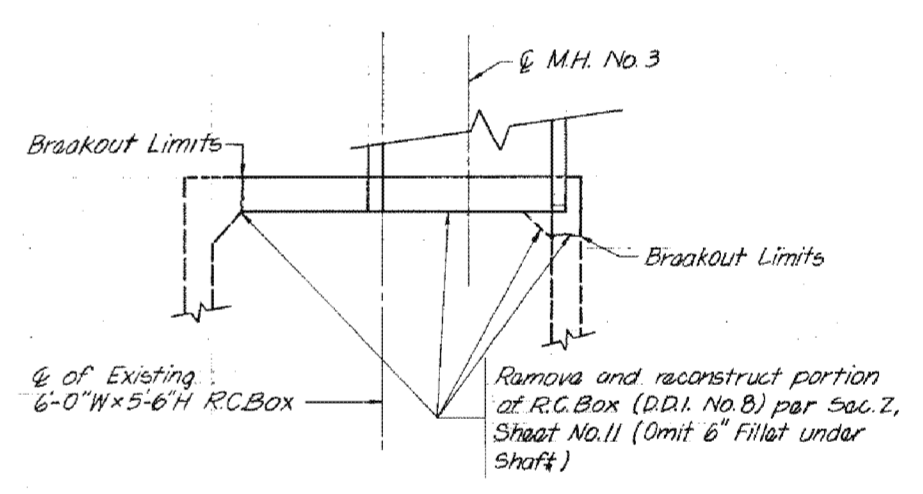
1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION SUBMITTED BY: <i>William M. Meyer</i> ASSISTANT CITY ENGINEER R.C.E. 17167	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT PROJECT NO. 4401 INGLEWOOD SINGLE R.C. BOX UNIT I SCHEDULE & DETAILS	
	APPROVED BY: <i>W. J. ...</i> CITY ENGINEER R.C.E. 11237	RECOMMENDED BY: <i>R. ...</i> DIVISION ENGINEER (DESIGN)

BOX SECTION		A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y		
Design Cover (ft.)		4.2'	7.8'	8.8'	10.1'	13.0'	10.1'	12.3'	9.1'	11.0'	9.2'	12.0'	9.5'	10.5'	9.7'	11.8'	10.9'	12.9'	15.1'	16.7'	14.3'	13.0'	11.1'	8.9'	9.1'	9.7'		
W	Width	7'-0"	10'-0"	8'-0"	8'-0"	11'-0"	8'-0"	11'-0"	8'-0"	11'-0"	8'-0"	11'-0"	8'-0"	9'-0"	7'-0"	9'-0"	7'-0"	9'-0"	7'-0"	9'-0"	15'-1"	16.7'	14.3'	13.0'	11.1'	8.9'	9.1'	9.7'
H	Height	12'-0"	9'-0"	11'-0"	11'-0"	8'-0"	11'-0"	8'-0"	11'-0"	8'-0"	11'-0"	8'-0"	11'-0"	7'-0"	9'-0"	7'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"	9'-0"
T1	Top Slab Thickness	6.50"	9.00"	8.25"	8.00"	12.50"	8.00"	12.00"	8.25"	11.00"	8.50"	11.75"	8.50"	9.00"	7.75"	9.75"	7.75"	8.50"	9.25"	12.00"	9.50"	9.00"	8.25"	7.75"	8.00"	8.25"	8.25"	
T2	Side Wall Thickness	3.00"	8.00"	8.00"	8.00"	8.75"	8.00"	8.50"	8.00"	8.25"	8.00"	8.50"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"	8.00"
T3	Bottom Slab Thickness	7.00"	10.00"	9.25"	9.25"	13.50"	9.25"	13.00"	9.25"	12.00"	9.50"	12.75"	9.75"	10.25"	8.75"	10.75"	8.75"	9.50"	10.25"	13.00"	10.50"	10.00"	9.25"	8.75"	8.15"	8.15"	9.25"	
B	Bar No. & Spacing	#6@17"	#9@16"	#8@18"	#6@11"	#8@10"	#6@11"	#5@10"	#8@18"	#7@8"	#8@18"	#7@8"	#8@18"	#8@15"	#7@15"	#7@10"	#7@17"	#8@13"	#9@20"	#9@13"	#9@20"	#7@13"	#8@19"	#6@11"	#8@13"	#9@17"		
Bars	Length, h	8'-1"	11'-1"	9'-1"	9'-1"	12'-25"	9'-1"	12'-2"	9'-1"	12'-15"	9'-1"	12'-2"	9'-1"	10'-1"	8'-1"	10'-1"	8'-1"	8'-1"	8'-1"	10'-25"	8'-7"	8'-7"	8'-7"	8'-7"	8'-7"	8'-7"	8'-7"	
B1	Bar No. & Spacing	#5@17"	#5@16"	#5@18"	#4@11"	#5@10"	#4@11"	#5@10"	#5@18"	#4@8"	#5@18"	#4@8"	#5@18"	#5@15"	#4@15"	#4@10"	#5@17"	#5@19"	#4@20"	#4@13"	#5@20"	#4@13"	#5@19"	#4@11"	#5@19"	#4@17"		
Bars	Length, h	5'-0.5"	5'-11.5"	5'-1"	5'-2"	7'-1.5"	5'-2"	6'-10.5"	5'-2"	6'-9"	5'-2"	7'-1"	5'-2.5"	5'-9"	4'-5"	5'-7"	4'-8.5"	4'-7.5"	4'-0.5"	5'-5"	4'-10.5"	4'-11"	4'-10.5"	4'-11"	4'-10.5"	4'-5.5"		
C	Bar No. & Spacing	#4@11"	#4@12"	#4@12"	#4@12"	#4@13"	#4@12"	#4@12"	#4@11"	#4@11"	#4@12"	#4@12"	#4@11"	#4@11"	#4@14"	#4@14"	#4@14"	#4@14"	#4@14"	#4@13"	#4@14"	#4@14"	#4@14"	#4@13"	#4@13"	#4@14"		
Bars	Horiz. Length, h	3'-5.5"	4'-2.5"	3'-8.5"	3'-8.5"	4'-6.5"	3'-8.5"	4'-6"	3'-8.5"	4'-6"	3'-8.5"	4'-6"	3'-8.5"	3'-11.5"	3'-5.5"	3'-11.5"	3'-5.5"	3'-5.5"	3'-5.5"	4'-0.5"	3'-7"	3'-7"	3'-7"	3'-7"	3'-7"	3'-7"		
	Vert. Length, v	11'-11.5"	9'-2"	11'-1.5"	11'-1"	8'-5.5"	11'-1"	8'-5"	11'-1.5"	8'-4"	11'-1.5"	8'-5"	11'-1.5"	7'-2"	9'-1"	7'-3"	9'-1"	9'-1.5"	9'-2.5"	7'-5"	9'-2.5"	9'-2"	9'-1.5"	9'-1"	9'-1.0"	9'-1.5"		
C1	Bar No. & Spacing	#5@11"	#5@12"	#6@12"	#6@12"	#5@13"	#6@12"	#5@12"	#5@11"	#5@11"	#6@12"	#5@12"	#5@11"	#4@11"	#5@14"	#5@14"	#5@14"	#5@14"	#5@14"	#4@13"	#5@14"	#5@14"	#5@14"	#5@13"	#5@13"	#5@14"		
Bars	Horiz. Length, h	2'-2.5"	1'-10"	1'-10"	1'-9"	2'-0.5"	1'-9"	1'-11.5"	1'-9"	1'-10"	1'-10"	1'-11"	1'-9.5"	1'-6.5"	1'-6.5"	1'-8"	1'-6.5"	1'-7"	1'-7.5"	1'-9"	1'-8"	1'-8"	1'-7"	1'-6.5"	1'-6.5"	1'-10"		
	Vert. Length, v	1'-7"	3'-5"	2'-1"	2'-0.5"	2'-11.5"	2'-0.5"	2'-11.5"	2'-0"	3'-1"	2'-1"	3'-0"	2'-0.5"	2'-6.5"	1'-9.5"	2'-11"	1'-9.5"	1'-10"	1'-10.5"	2'-2"	1'-11.5"	1'-11.5"	1'-11"	1'-10.5"	1'-10.5"	1'-11"		
C2	Bar No. & Spacing	#5@11"	#5@12"	#4@12"	#4@12"	#4@13"	#4@12"	#4@12"	#4@11"	#4@11"	#4@12"	#4@11"	#4@11"	#4@14"	#4@14"	#4@14"	#4@14"	#4@14"	#4@14"	#4@13"	#4@14"	#4@14"	#4@14"	#4@13"	#4@13"	#4@14"		
Bars	Horiz. Length, h	3'-9"	4'-3.5"	3'-8.5"	3'-8.5"	4'-6.5"	3'-8.5"	4'-6"	3'-8.5"	4'-6"	3'-8.5"	4'-6"	3'-8.5"	3'-11.5"	3'-5.5"	3'-11.5"	3'-5.5"	3'-5.5"	3'-5.5"	4'-0.5"	3'-7"	3'-7"	3'-7"	3'-7"	3'-7"	3'-7"		
	Vert. Length, v	2'-0"	2'-3"	2'-2.5"	2'-2.5"	2'-6.5"	2'-2.5"	2'-6"	2'-2.5"	2'-5"	2'-2.5"	2'-6"	2'-3"	2'-3.5"	2'-2"	2'-4"	2'-2"	2'-2.5"	2'-3.5"	2'-6"	2'-3.5"	2'-3"	2'-2.5"	2'-2"	2'-2.0"	2'-2.5"		
C3	Bar No. & Spacing	#5@11"	#4@12"	#6@12"	#6@12"	#4@13"	#6@12"	#4@12"	#6@11"	#4@11"	#6@12"	#4@12"	#5@11"	#4@11"	#5@14"	#4@14"	#5@14"	#4@13"	#5@14"	#4@13"	#5@14"	#5@14"	#5@13"	#5@13"	#5@14"	#5@14"		
Bars	Horiz. Length, h	1'-8.5"	1'-6"	2'-1"	2'-1"	1'-11"	2'-1"	1'-10"	1'-10.5"	1'-8.5"	2'-0.5"	1'-10"	1'-11"	1'-6"	1'-1.5"	1'-7.5"	1'-8"	1'-8.5"	1'-8"	1'-8"	1'-9"	1'-8.5"	1'-8"	1'-7.5"	1'-7.5"	1'-8"		
	Vert. Length, v	1'-3"	1'-11.5"	2'-0"	2'-0.5"	2'-6.5"	2'-0.5"	2'-7.5"	2'-0"	2'-3.5"	2'-0.5"	2'-9"	2'-0"	2'-4.5"	1'-9.5"	5'-7.5"	1'-9.5"	1'-10.5"	1'-11"	1'-10.5"	1'-10.5"	1'-11"	1'-10.5"	1'-10"	1'-10"	1'-10.5"		
D	Bar No. & Spacing	#5@9"	#4@18"	#4@9"	#4@9"	#4@18"	#4@9"	#4@18"	#4@9"	#4@18"	#4@9"	#4@18"	#4@9"	#4@18"	#4@17"	#4@18"	#4@17"	#4@18"	#4@18"	#4@18"	#4@18"	#4@18"	#4@17"	#4@17"	#4@17"	#4@17"		
Bars	Length, v	12'-10.5"	10'-4"	12'-2.5"	12'-2.5"	9'-11"	12'-2.5"	9'-10"	12'-2.5"	9'-8"	12'-3"	9'-9.5"	12'-3"	8'-4"	10'-1.5"	8'-5.5"	10'-1.5"	10'-3"	10'-4.5"	8'-10"	10'-5"	10'-4"	10'-2.5"	10'-1.5"	10'-1.5"	10'-2.5"		
F	Bar No. & Spacing	#7@17"	#8@11"	#9@19"	#9@19"	#9@11"	#9@19"	#9@11"	#8@14"	#9@12"	#8@14"	#8@9"	#8@14"	#9@16"	#7@13"	#9@15"	#7@13"	#9@20"	#9@18"	#9@12"	#9@16"	#8@14"	#7@12"	#9@20"	#9@20"	#9@20"		
Bars	Length, h	8'-1"	11'-1"	9'-1"	9'-1"	12'-2.5"	9'-1"	12'-2"	9'-1"	12'-1.5"	9'-1"	12'-2"	9'-1"	10'-1"	8'-1"	10'-1"	8'-1"	8'-1"	8'-1"	10'-2.5"	8'-7"	8'-7"	8'-7"	8'-7"	8'-7"	8'-7"		
F1	Bar No. & Spacing	#5@17"	#4@11"	#5@19"	#5@19"	#5@11"	#5@19"	#5@11"	#4@14"	#5@12"	#4@14"	#4@9"	#4@14"	#5@16"	#4@13"	#5@15"	#4@13"	#5@20"	#4@18"	#4@12"	#4@16"	#4@14"	#4@12"	#5@20"	#5@20"	#5@20"		
Bars	Length, h	4'-11"	6'-0.5"	5'-3.5"	5'-3"	7'-1"	5'-3"	6'-9.5"	5'-0"	7'-1.5"	4'-11.5"	7'-1"	5'-0.5"	5'-11"	4'-9"	6'-0"	4'-8.5"	4'-9"	4'-6"	5'-9.5"	4'-5.5"	4'-11"	5'-1.5"	4'-11.5"	5'-0"	5'-1.5"		
G	Bar No. & Spacing	#4@11"	#4@12"	#4@12"	#4@12"	#4@13"	#4@12"	#4@12"	#4@11"	#4@11"	#4@12"	#4@12"	#4@11"	#4@11"	#4@14"	#4@14"	#4@14"	#4@14"	#4@13"	#4@14"	#4@14"	#4@14"	#4@14"	#4@13"	#4@13"	#4@14"		
Bars	Length, h	3'-6"	5'-0"	4'-0"	4'-0"	5'-6"	4'-0"	5'-6"	4'-0"	5'-6"	4'-0"	5'-6"	4'-0"	4'-6"	3'-6"	4'-6"	3'-6"	3'-6"	3'-6"	4'-6"	3'-9"	3'-9"	3'-9"	3'-9"	3'-9"	3'-9"		
H	Bar No. & Spacing	#5@11"	#4@12"	#4@12"	#4@13"	#4@12"	#4@12"	#4@12"	#4@11"	#4@11"	#4@12"	#4@12"	#4@11"	#4@11"	#4@14"	#4@14"	#4@14"	#4@14"	#4@13"	#4@14"	#4@14"	#4@14"	#4@13"	#4@13"	#4@14"	#4@14"		
Bars	Length, h	3'-6"	5'-0"	4'-0"	4'-0"	5'-6"	4'-0"	5'-6"	4'-0"	5'-6"	4'-0"	5'-6"	4'-0"	4'-6"	3'-6"	4'-6"	3'-6"	3'-6"	3'-6"	4'-6"	3'-9"	3'-9"	3'-9"	3'-9"	3'-9"	3'-9"		
		NUMBER OF LONGITUDINAL REINFORCEMENT #4 BARS													NO. OF LONG. REINF. #4 BARS													
Top Slab (Includes Distrib.)		13	17	14	14	19	14	19	14	19	14	19	14	16	13	16	13	13	13	16	14	14	14	14	14	14		
Bottom Slab		13	16	14	14	19	14	19	14	19	14	19	14	16	13	16	13	13	13	16	14	14	14	14	14	14		
Side Walls		32	24	28	28	20	28	20	28	20	28	20	28	16	24	16	24	24	24	16	24	24	24	24	24	24		
TOTAL		58	57	56	56	58	56	58	56	58	56	58	56	48	50	48	50	50	50	48	52	52	52	52	52	52		
		QUANTITIES													QUANTITIES													
Concrete Cu. Yds./Lin. Ft.		0.96	1.13	1.07	1.06	1.46	1.06	1.40	1.07	1.31	1.08	1.38	1.09	0.98	0.89	1.02	0.89	0.93	0.97	1.21	1.01	0.98	0.94	0.92	0.92	0.94		
Steel Lbs./Lin. Ft.		159.5	167.0	158.5	157.2	195.1	157.2	197.7	159.6	194.0	159.6	191.6	156.2	139.3	113.4	142.6	112.9	116.3	118.6	150.2	127.0	124.0	120.9	122.8	123.0	121.2		
		LINE A													LINE B													



DETAILS OF REMOVAL AND RECONSTRUCTION OF A PORTION OF D.D.I. No. 8 IN 107TH STREET (2A) (REFER TO SHEET No. 6)



NOTE: Refer to Sheet 3 for Concrete Breakout Notes

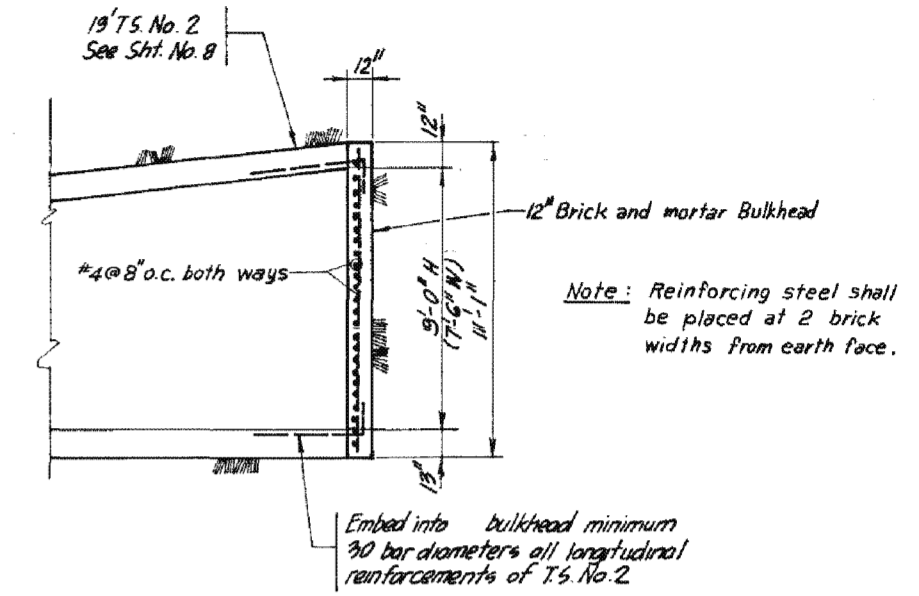
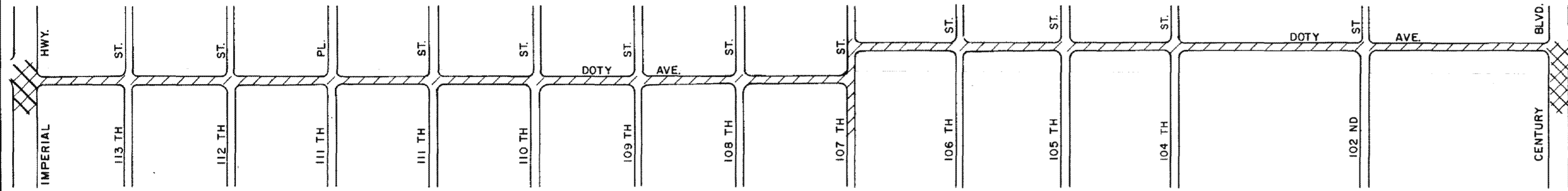


1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
	PROJECT NO. 4401 INGLEWOOD LINE A UNIT I	
SUBMITTED BY: <i>William M. Mahan</i> ASSISTANT CITY ENGINEER R.C.E. 17167	SINGLE R.C. BOX SCHEDULE	
APPROVED BY: <i>William J. Lawrence</i> CITY ENGINEER R.C.E. 11237 DATE 1-31-74	RECOMMENDED BY: <i>William M. Mahan</i> DIVISION ENGINEER (DESIGN)	
SCALE AS SHOWN	DATE SEPT 74	DWG. NO. 364-4401-06.12 SHEET 12 OF 14

REVISIONS			
MARK	DATE	DESCRIPTION	

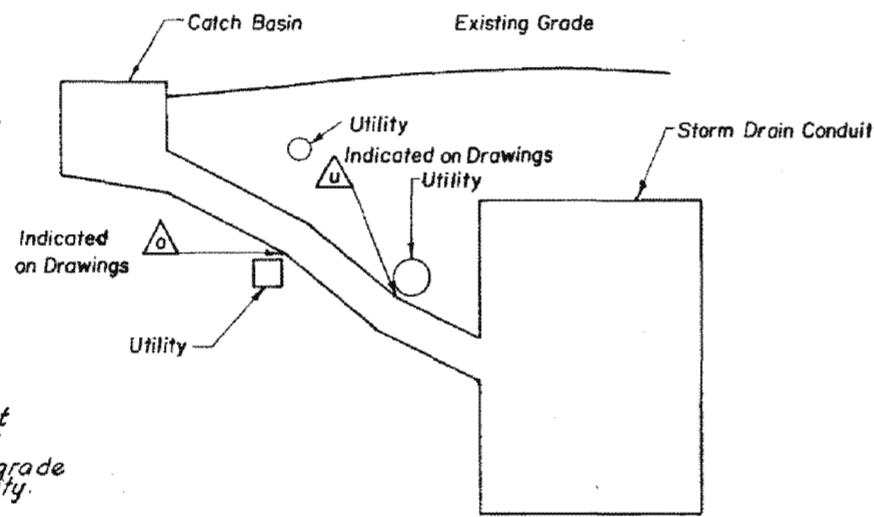
"AS BUILT" DRAWING



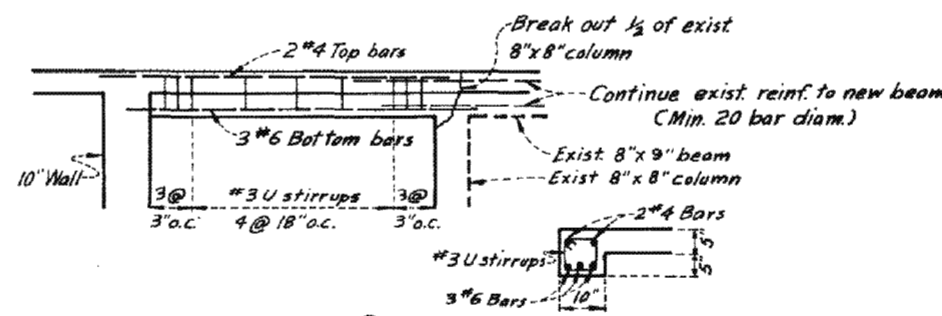
BULKHEAD DETAIL (Refer to sheet No. 8)
SCALE: 1/4" = 1'-0"

NOTES:

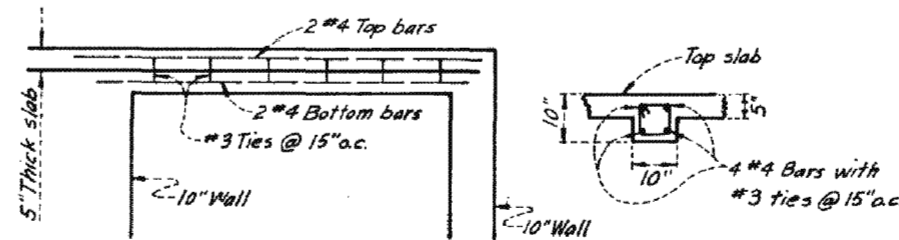
1. The change in grade of the connector pipe may occur either over or under an existing utility. The utilities at which the grade change occur are shown on the project drawings. At locations where utility crossings are marked Δ the connector pipe will break over the utility. At locations where the utility crossings are marked \triangle the connector pipe will break under the utility.
2. On those connector pipes where change of grade is not indicated, it is assumed that the connector pipe can be laid on a straight grade from the catch basin to the storm drain without interference with utilities.
3. Where connector pipe has a change of grade exceeding 1/10' FT or differs in diameter from that of existing pipe, use concrete collar per standard DWG 2-D393.
4. The Contractor shall make exploratory excavation to determine the exact location and depth of utilities except sanitary sewers which are marked Δ or \triangle . After the exact location of a utility has been determined, the grade and alignment of the connector pipe will be staked so as to clear the utility.



TYPICAL CONNECTOR PIPE PROFILE
NO SCALE



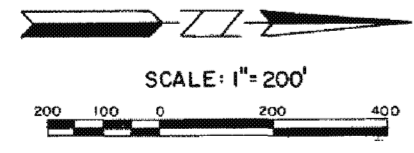
DETAIL 1
NOT TO SCALE



DETAIL 2
NOT TO SCALE

Pavement Reconstruction Legend

Existing Section	Reconstruction Section
3" A.C. over 6" A.B.	4" A.C. over 6" C.A.B.
2" A.C. over 4" Base	3" A.C. over 4" C.A.B.
6" A.C. over 8" A.B.	6" A.C. over 8" C.A.B.
5" A.C. over 12" A.B.	6" A.C. over 12" C.A.B.



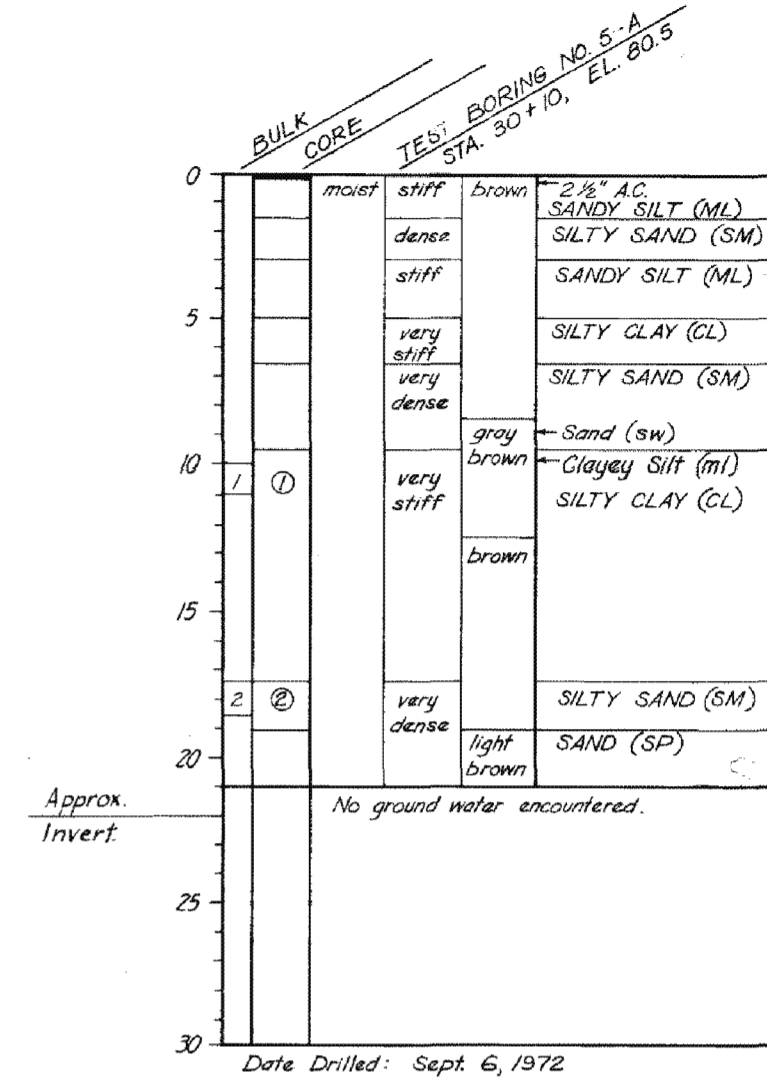
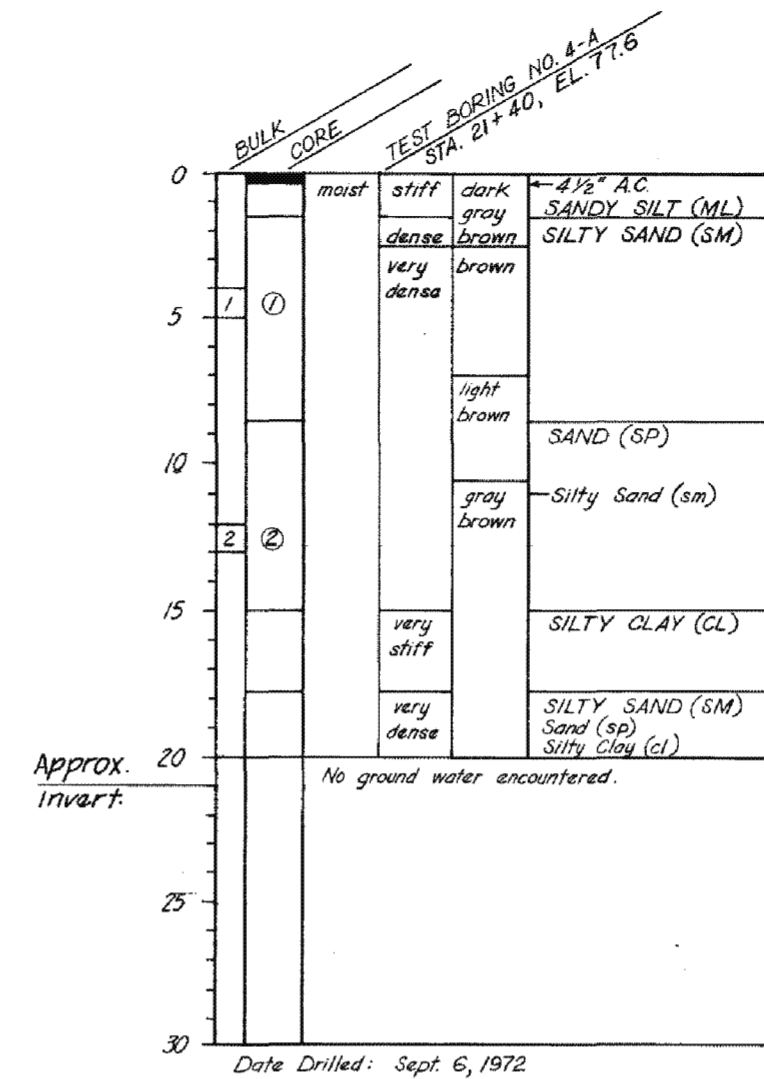
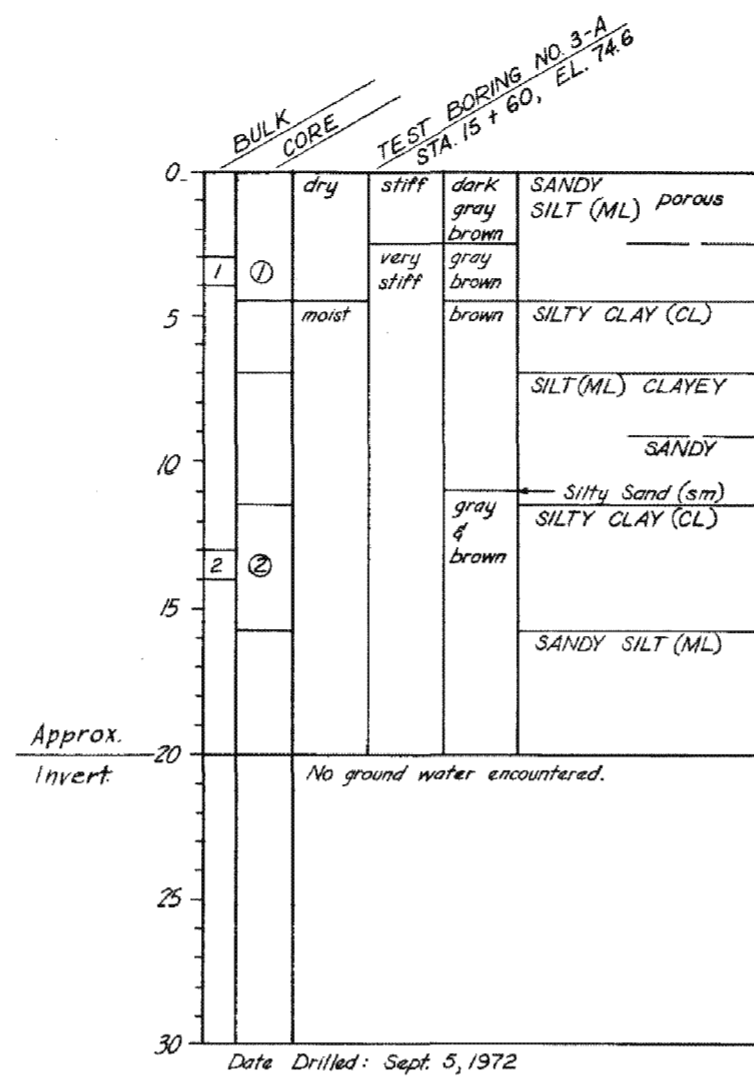
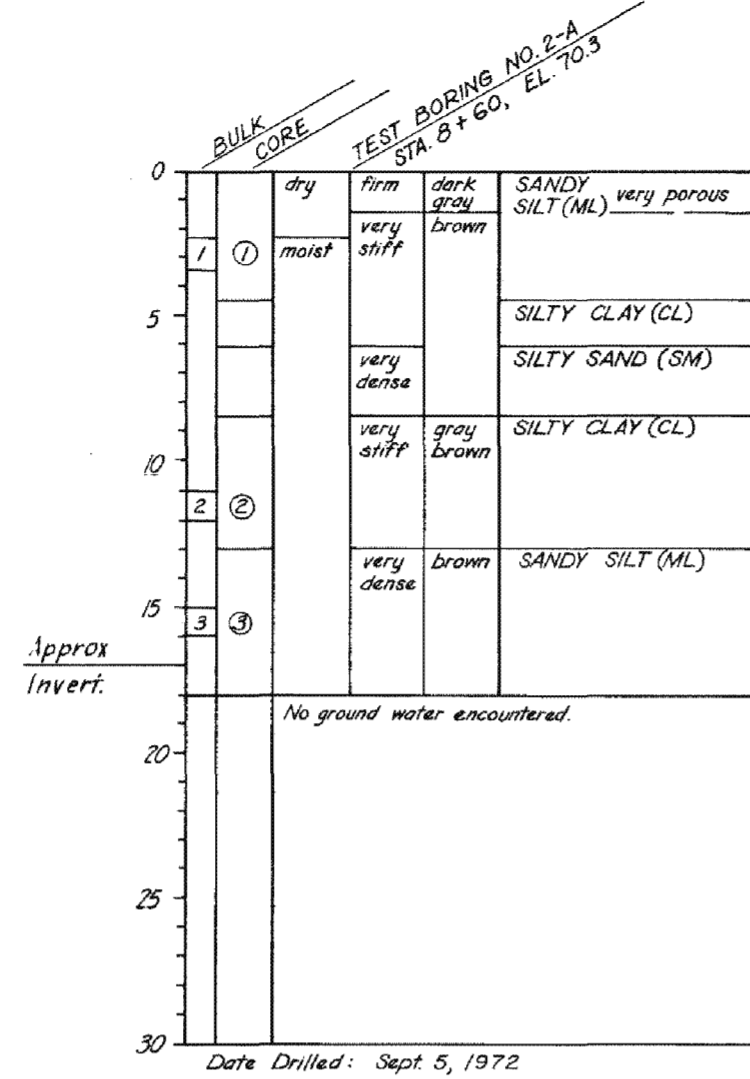
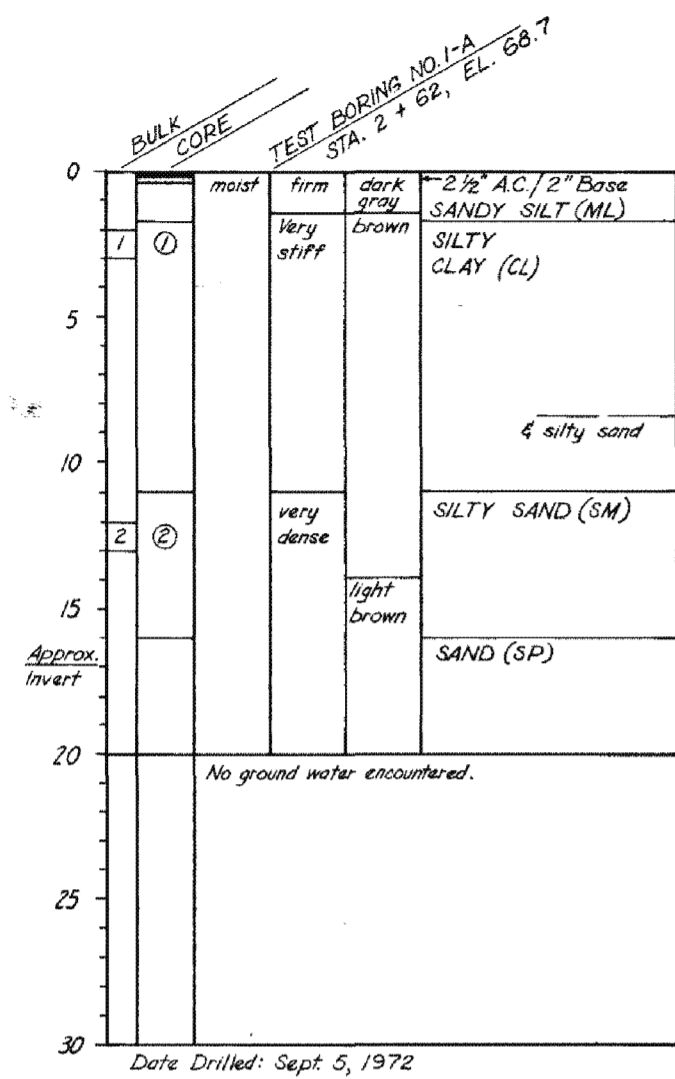
REVISIONS		
MK	DATE	DESCRIPTION
1	12-9-74	Addition of Details



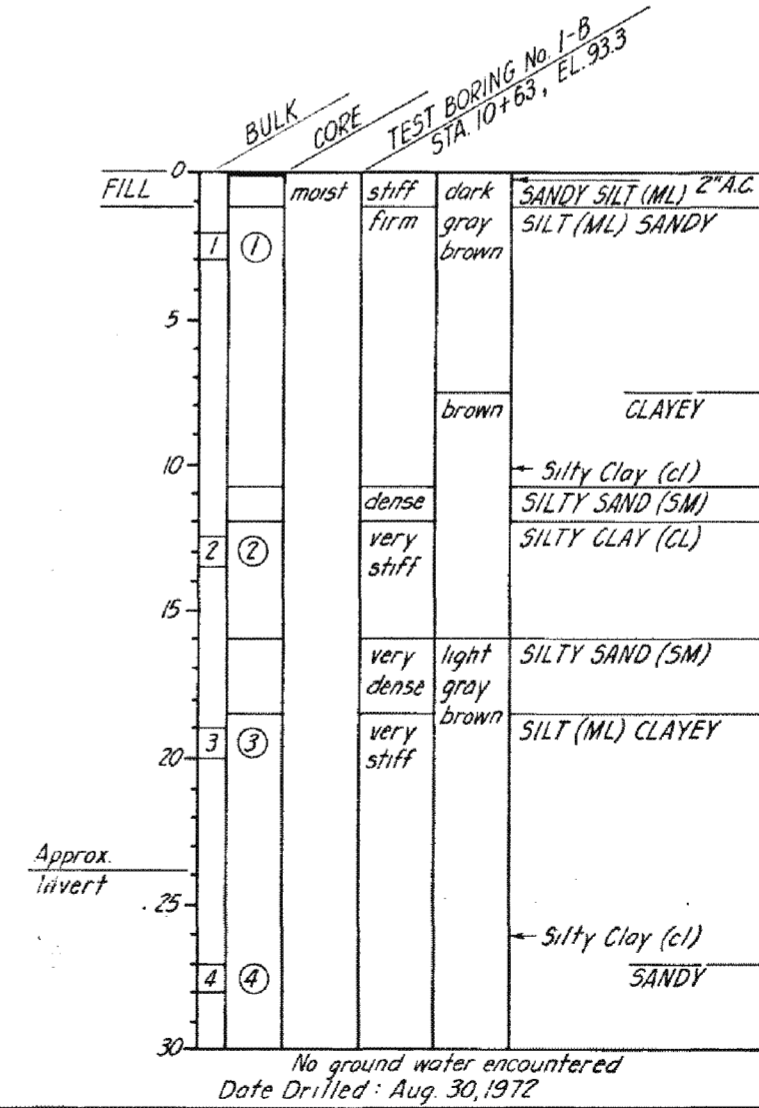
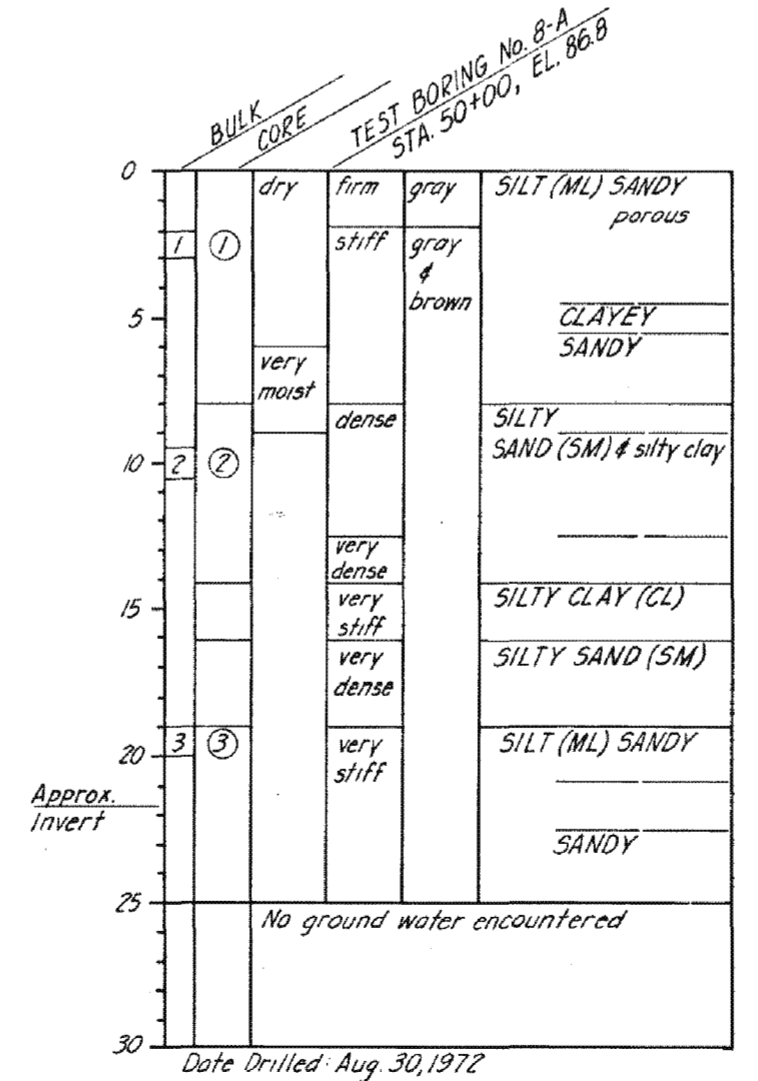
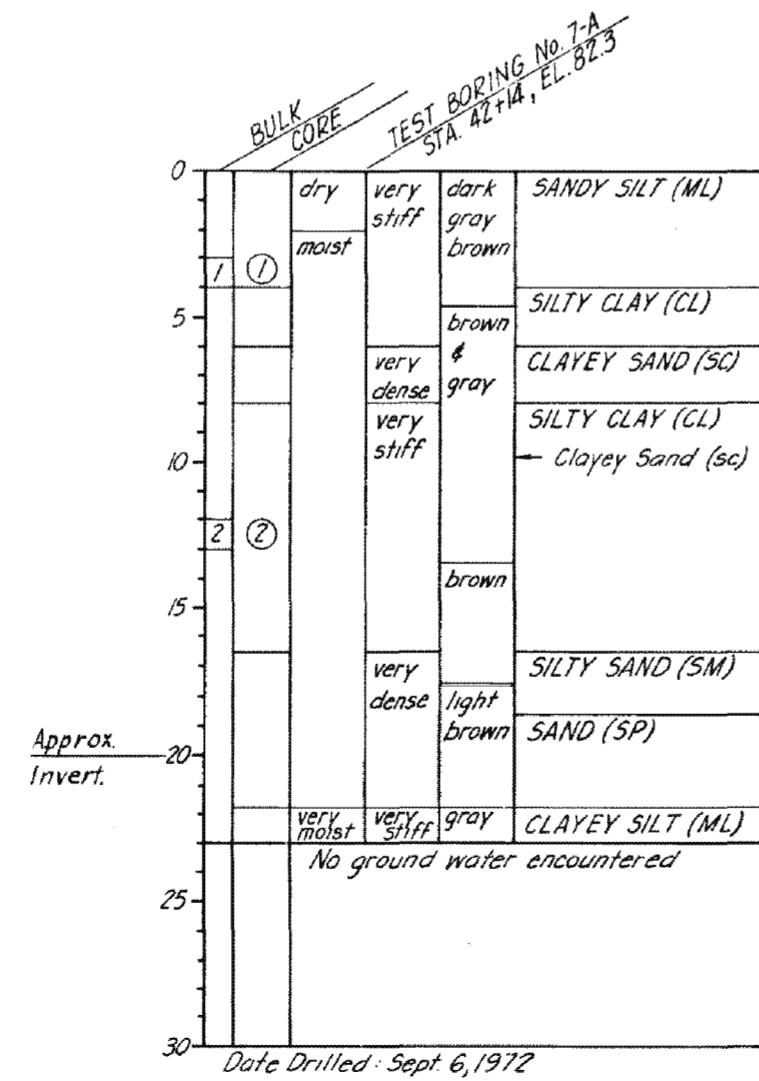
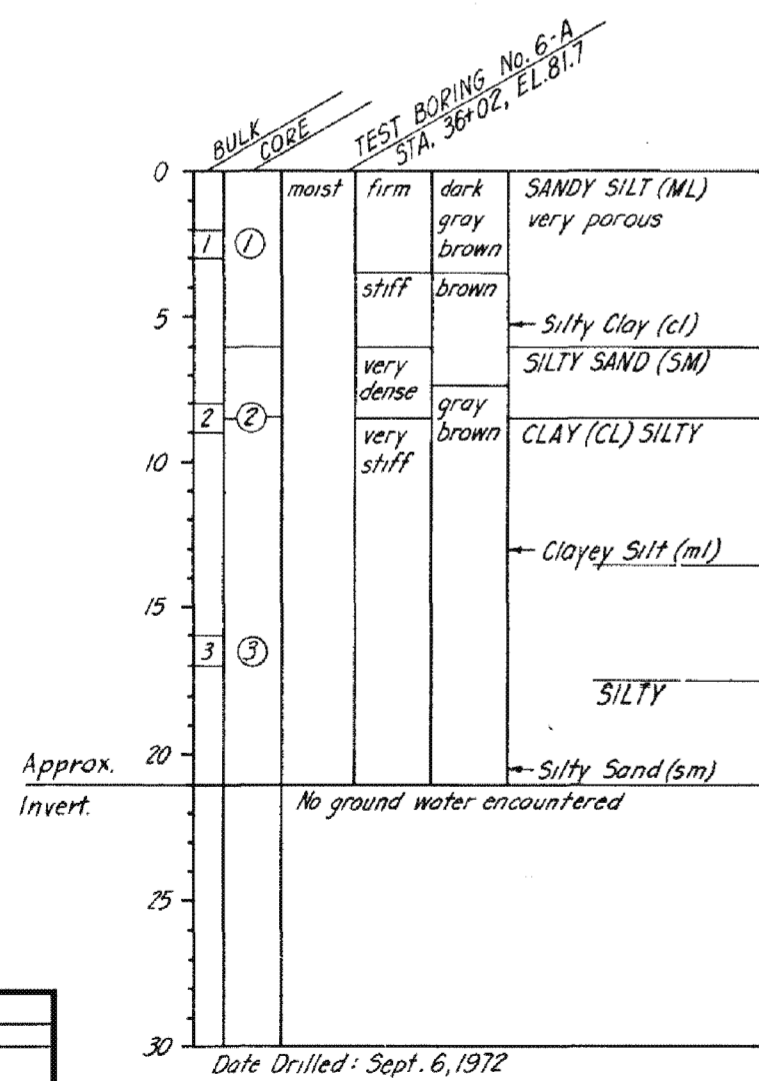
1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: <i>William M. ...</i> ASSISTANT CITY ENGINEER R.C.E. 17167		PROJECT NO. 4401 INGLEWOOD RESURFACING PLAN UNIT I	
APPROVED BY: <i>W. ...</i> CITY ENGINEER R.C.E. 11237	DATE: 1.31.74	RECOMMENDED BY: <i>R. ...</i> DIVISION ENGINEER (DESIGN)	DATE: SEPT 74 SCALE AS SHOWN NO. 364-4401-D6.13 SHEET 13 OF 14

"AS BUILT" DRAWING



- NOTES:**
- Soil classification was made in accordance with the Unified Soil Classification System shown on Std. Dwg. No. 2-D413.
 - Borings were drilled with an 18" diameter Truck-Mounted Rotary Bucket Auger.
 - Approximate invert elevations shown above Supersedes Soils Report.



REVISIONS		
MK	DATE	DESCRIPTION

CITY OF INGLEWOOD
PUBLIC WORKS DEPARTMENT
ENGINEERING DIVISION

SUBMITTED BY:
William Maher
ASSISTANT CITY ENGINEER R.C.E. 17167

APPROVED BY:
John J. ...
CITY ENGINEER R.C.E. 11237 DATE 1-31-74



1964 STORM DRAIN BOND ISSUE

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

PROJECT NO. 4401
INGLEWOOD
LINE A UNIT I
LOG OF BORINGS

RECOMMENDED BY: *Rymilne* DATE SEPT 74 SCALE AS SHOWN
DIVISION ENGINEER (DESIGN) NO. 364-4401-D 6 14

SHEET 4 OF 14

AS BUILT DRAWING

LEGEND & ABBREVIATIONS

PROPERTY LINE	—
CURB	—
GUTTER	—
SIDE INLET CATCH BASIN	—
EXISTING DRAINAGE STRUCTURES	—
GRATING TYPE CATCH BASIN	—
DRIVEWAY	—
LIMIT OF CONCRETE SURFACE	—
WALK	—
TRAFFIC SIGNAL	—
DEAD MAN	—
POWER, TELEPHONE OR GUY POLE	—
FIRE HYDRANT	—
ELECTROLIER	—
WATER & GAS METER	—
BENCH MARK	—
TEST BORING	—
TREE	—
TREES TO BE REPLACED BY CONTRACTOR	—
UNDER EXISTING UTILITY	—
OVER EXISTING UTILITY	—
TRAFFIC SIGNAL CONDUIT	T.S.C.
TRANSITION STRUCTURE	T.S.
MANHOLE	M.H.
LOCAL DEPRESSION	L.D.
REINFORCED CONCRETE PIPE	R.C.P.
TEST BORING	T.B.
JUNCTION STRUCTURE	J.S.
CATCH BASIN	C.B.
SANITARY SEWER	S.S.
TEMPORARY BENCH MARKS	T.B.M.
SOUTHERN CALIFORNIA GAS CO.	S.C.G.
L.A. DEPT. OF WATER & POWER WATER SYST. - DW&PWS	S.C.W.
SOUTHERN CALIFORNIA WATER CO.	S.C.W.
INGLEWOOD CITY WATER DEPT.	ING. W.
SOUTHERN CALIFORNIA EDISON CO.	S.C.E.
PACIFIC TELEPHONE & TELEGRAPH CO.	P.T.&T.

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

STANDARD DRAWINGS

DRAWING NO.	TITLE
2-D88	LOCAL DEPRESSION NO. 2
2-D96	STANDARD DROP STEP
2-D107	CONCRETE RINGS, REDUCER AND PIPE
2-D193	JUNCTION STRUCTURE NO. 4
2-D156	MANHOLE FRAME AND COVER FOR CATCH BASINS
2-D163	CATCH BASIN NO. 3 (W=10', 14', 21', 28', ETC.)
2-D172	CATCH BASIN REINFORCEMENT
2-D173.1 TO 3	PIPE SUPPORTS ACROSS TRENCHES
2-D175	REMOVABLE PROTECTION BAR FOR CATCH BASINS
2-D177	PIPE BEDDING IN TRENCHES
2-D181	STANDARD NON-ROCKING MANHOLE FRAME AND COVER
2-D184	MANHOLE NO. 2 (PIPES, 36" OR LARGER)
2-D213.1 & 2	"D" LOAD TABLE FOR DESIGN OF REINFORCED CONCRETE PIPE
2-D171	STANDARD A-305 REINFORCING BARS
2-D232	DETAIL OF CATCH BASIN OPENING
2-D250	REMODELING OF SANITARY SEWER HOUSE CONNECTIONS
2-D251	PROTECTION FOR MAIN LINE AND HOUSE CONNECTION SEWERS
2-D109	CATCH BASIN NO. 6
2-D224	CONNECTION TO CATCH BASIN FOR PIPES 12" THROUGH 72"
2-D227	FRAME AND GRATING FOR CATCH BASINS
2-D264	ADJUSTABLE PROTECTION BAR STIRRUP
2-D393	CONCRETE COLLAR FOR PIPES 12" THROUGH 66"
2-D413	UNIFIED SOIL CLASSIFICATION SYSTEM
2-D170	CATCH BASIN NO. 7
2-D399	CRITERIA FOR THE DESIGN OF SHORING FOR EXCAVATIONS
2-D400	SAMPLE SHEET FOR USE AS A GUIDE IN PREPARING CALCULATIONS FOR SHORING OF EXCAVATIONS
2-D113	MANHOLE NO. 4
2-D112	JUNCTION STRUCTURE NO. 2
2-D160	CATCH BASIN NO. 1

GENERAL NOTES

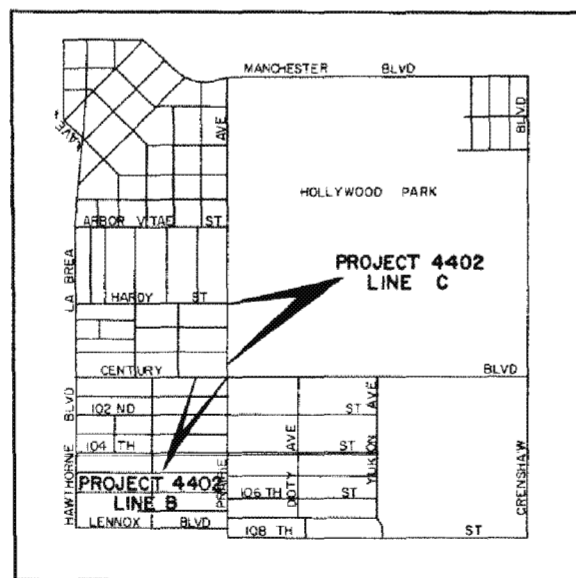
- NUMBERS IN CIRCLES INDICATE ITEMS UNDER WHICH PAYMENT WILL BE MADE.
- ELEVATIONS SHOWN ARE IN FEET ABOVE THE U.S.G.S. MEAN SEA LEVEL DATUM.
- STATIONS SHOWN ON DRAWINGS ARE ALONG CENTER LINE OF CONDUIT OR ON A LINE NORMAL TO CENTER LINE OF CONDUIT.
- STATIONS AND INVERT ELEVATIONS OF PIPE INLETS SHOWN ON THE PROFILES ARE AT THE INSIDE FACE OF THE CONDUIT, UNLESS OTHERWISE SHOWN.
- ALL FIELD BOOK REFERENCES ARE TO CITY OF INGLEWOOD FIELD BOOKS, UNLESS OTHERWISE NOTED.
- ALL PIPE IN OPEN TRENCH SHALL BE BEDDED ACCORDING TO STANDARD DRAWING 2-D177, CASE III, UNLESS OTHERWISE SHOWN OR MODIFIED IN THE SPECIFICATIONS.
- PIPE CONNECTIONS TO STORM DRAIN SHALL CONFORM TO STANDARD DRAWING 2-D193, UNLESS OTHERWISE SHOWN.
- TIES FOR CATCH BASINS AS SHOWN ON THE DRAWINGS ARE FROM CURB RETURN TO CENTER LINE OF CATCH BASIN, UNLESS OTHERWISE SHOWN.
- LOCATIONS OF CATCH BASIN CONNECTOR PIPE JUNCTIONS WITH CATCH BASINS AS SHOWN ON THE DRAWINGS ARE SCHEMATIC. IT IS INTENDED THAT SUCH JUNCTIONS BE LOCATED AT THE DOWNSTREAM ENDS OF THE CATCH BASINS, UNLESS OTHERWISE SHOWN. IN ALL CASES THE EXACT LOCATIONS WILL BE DETERMINED IN THE FIELD BY THE ENGINEER TO MEET FIELD CONDITIONS.
- MONOLITHIC CATCH BASIN CONNECTIONS SHALL BE CONSTRUCTED, WHERE APPLICABLE, PER STANDARD DRAWING 2-D224.
- "V_i" IS THE DEPTH OF INLET OF CATCH BASINS IN SERIES MEASURED FROM TOP OF CURB TO INVERT OF CONNECTOR PIPE.
- ALL EXISTING SANITARY SEWERS SHOWN ON THE DRAWINGS ARE CITY OF INGLEWOOD SEWERS.
- EXISTING UTILITIES SHALL BE MAINTAINED IN PLACE BY THE CONTRACTOR, UNLESS OTHERWISE NOTED.
- WHERE UTILITIES ARE INDICATED ON THE DRAWINGS TO BE SUPPORTED, SAID SUPPORTS SHALL BE IN ACCORDANCE WITH STANDARD DRAWING 2-D173.1, 2 OR 3, UNLESS OTHERWISE INDICATED.
- LOCATIONS SHOWN ON THE PLANS FOR EXISTING SANITARY SEWER HOUSE CONNECTIONS ARE APPROXIMATE ONLY.
- SANITARY SEWER HOUSE CONNECTION RECONSTRUCTION AND RECONNECTION SHALL BE IN ACCORDANCE WITH STANDARD DRAWING 2-D250, UNLESS OTHERWISE SHOWN.
- SANITARY SEWERS AND HOUSE CONNECTIONS CROSSING OVER THE STORM DRAIN TRENCH SHALL BE SUPPORTED IN ACCORDANCE WITH STANDARD DRAWINGS 2-D173.1 TO 3 AND ENCASED PER GENERAL NOTE 1 ON STANDARD DRAWING 2-D173.1.
- WHEN INDICATED ON THE DRAWINGS, SANITARY SEWERS AND HOUSE CONNECTIONS SHALL BE ENCASED OR BLANKETED IN ACCORDANCE WITH STANDARD DRAWING 2-D251.
- ALL RESURFACING, CURBS, GUTTERS, SIDEWALKS, DRIVEWAYS AND OTHER EXISTING IMPROVEMENTS TO BE RECONSTRUCTED SHALL BE CONSTRUCTED AT THE SAME ELEVATION AND LOCATION AS THE EXISTING IMPROVEMENTS, UNLESS OTHERWISE NOTED.
- SOIL TEST BORINGS FOR THIS PROJECT WERE MADE 8-26-65 AND 8-27-65
- THE DEPTH AT THE UPSTREAM END OF CATCH BASINS 10 FEET OR MORE IN LENGTH SHALL BE CURB FACE PLUS 12 INCHES UNLESS OTHERWISE SHOWN.
- REFER TO SHEET 4 FOR TYPICAL CATCH BASIN CONNECTOR PIPE PROFILE.
- ALL OPENINGS RESULTING FROM THE CUTTING OR PARTIAL REMOVAL OF EXISTING PIPES, CULVERTS, OR SIMILAR STRUCTURES SHALL BE SEALED WITH 8 INCHES OF BRICK AND MORTAR OR 6 INCHES OF CONCRETE.
- THE OPENINGS AND TOP SLABS OF ALL SIDE INLET CATCH BASINS AND CATCH BASINS NO. 6 AND 7 SHALL BE MODIFIED TO MEET THE REQUIREMENTS SHOWN ON STANDARD DRAWING 2-D232.
- WHERE REQUIRED BY STANDARD DRAWING 2-D213.1, CONCRETE BACKFILL SHALL BE USED AROUND CONNECTOR PIPES 36 INCHES OR LESS IN DIAMETER. CONCRETE BACKFILL FOR MAIN LINE PIPE SHALL BE USED ONLY WHEN DIRECTED BY THE ENGINEER.
- THE OPENING OF ALL SIDE INLET CATCH BASINS AND CATCH BASIN NO. 6 THAT HAVE MODIFIED LOCAL DEPRESSIONS TO BE CURB FACE PLUS 1" IN LIEU OF 9".
- CONSTRUCT LOCAL DEPRESSION PER STANDARD DRAWING 2-D109 AND AS MODIFIED ON THE DRAWINGS FOR CATCH BASIN NO. 6.

INDEX TO DRAWINGS

SHEET NO.	TITLE
1.	LOCATION MAP, GENERAL NOTES, INDEX TO DRAWINGS, STANDARD DRAWINGS, ABBREVIATIONS AND LEGEND.
2.	LINE C, PLAN, PROFILE AND SECTION STA 25+00 TO STA 34+85
3.	LINE B, PLAN, PROFILE AND SECTION STA 39+80 TO STA 46+66
4.	LOG OF BORINGS AND RESURFACING PLAN.

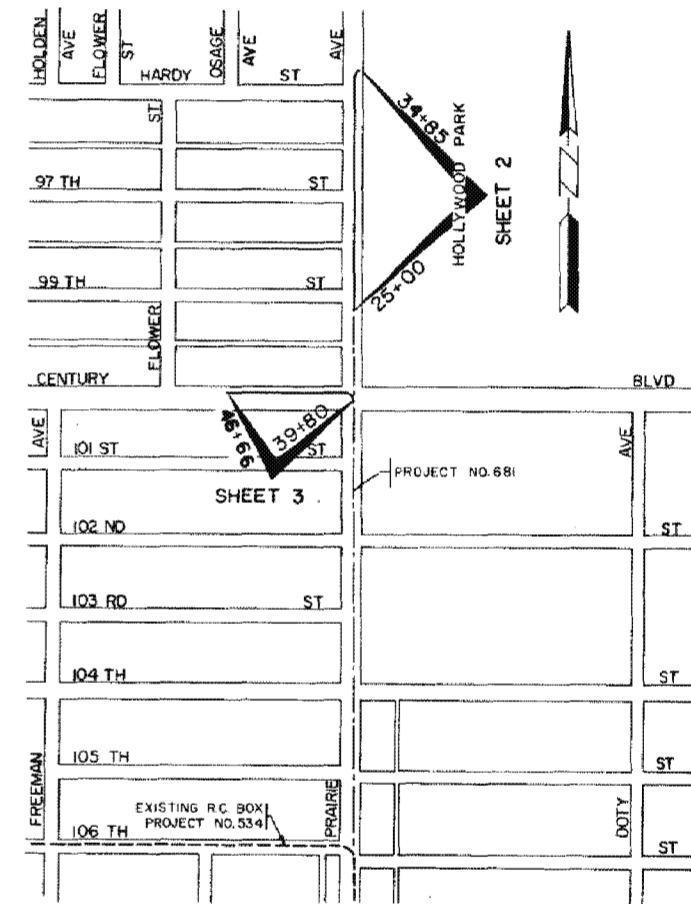
LOS ANGELES COUNTY ROAD DEPARTMENT
STANDARD DRAWINGS

DRAWING NO.	TITLE
M57-39R	PARTIAL CONCRETE REPLACEMENT FOR CROSS GUTTERS AND SPANDRELS
M57-45R	SPECIFICATIONS FOR REPAIRS OF CUTS IN CONCRETE PAVEMENT



VICINITY MAP
1000 0 1000 2000 3000 4000
GRAPHIC SCALE

NOTE CHANGE OF PROJECT LIMITS
Due to bond issue financing requirements, the upstream limit of Project 681, has been changed to Station 14+98. See DWG. NO. 364-681-D23. The downstream limit of Project 4402 Unit I, Line C, has been extended to meet the new upstream limit of Project 681, see DWG NO. 364-4402-D22.



LOCATION MAP

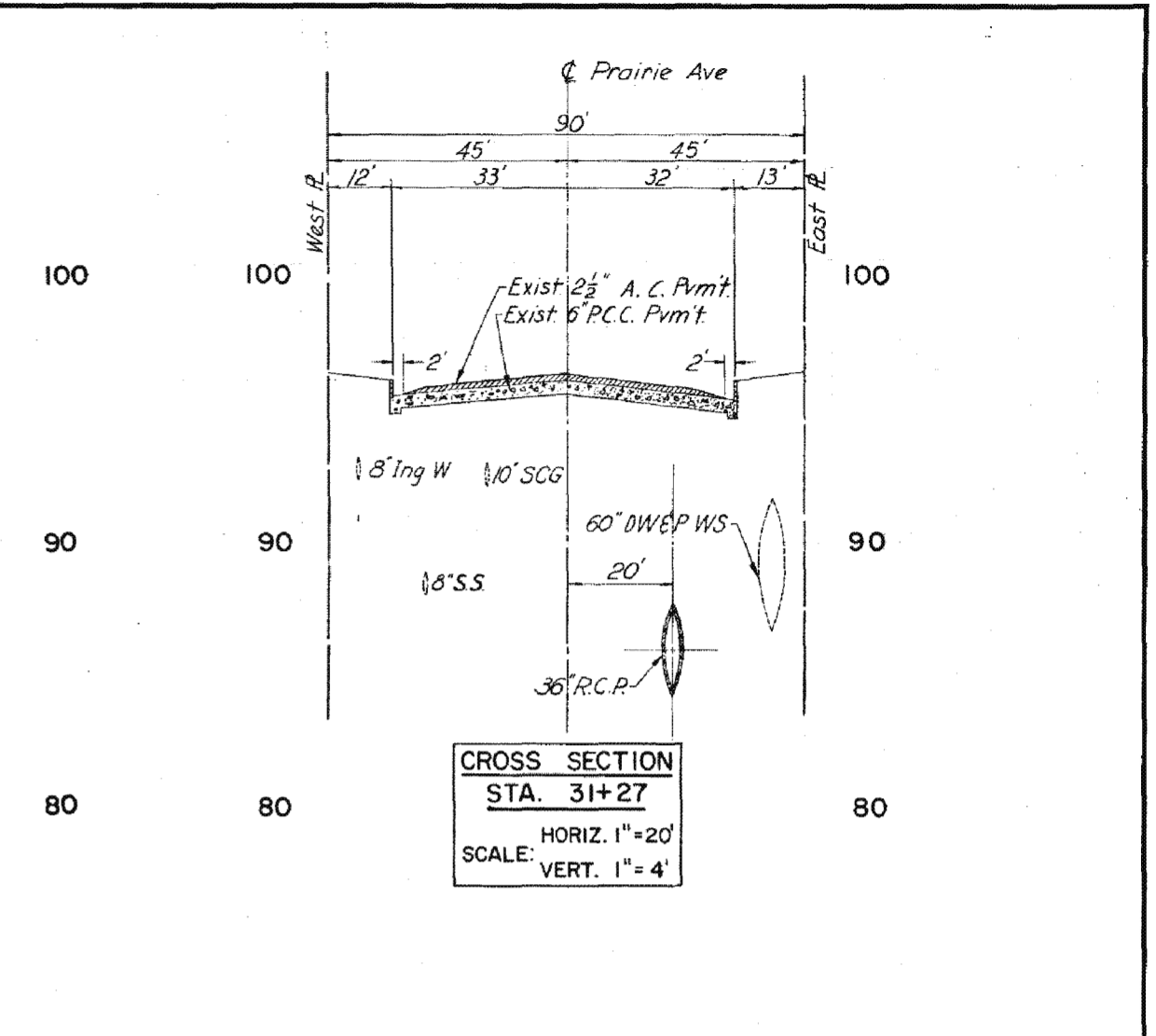
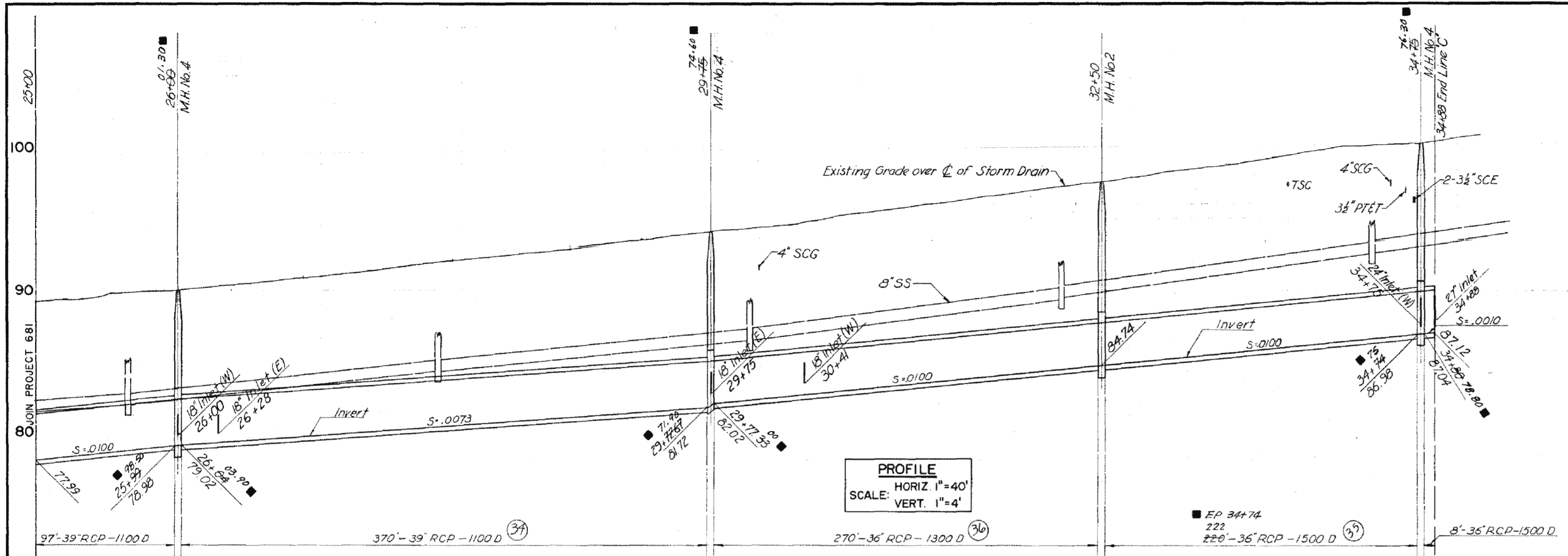
DATE	REVISIONS
5-4-67	Note Change Of Project Limits

"AS BUILT DWGS."



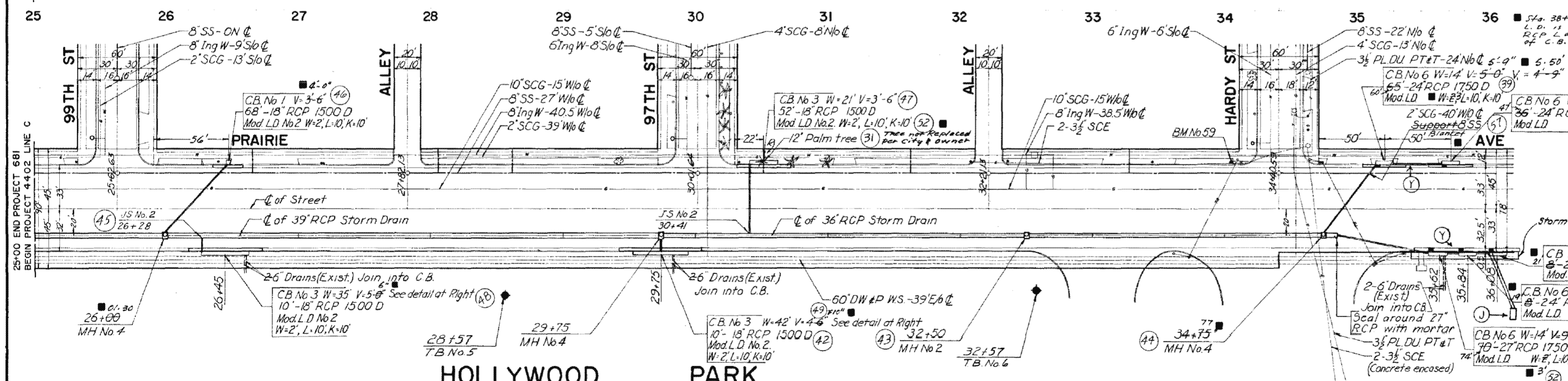
CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		1964 STORM DRAIN BOND ISSUE LOS ANGELES COUNTY FLOOD CONTROL DISTRICT PROJECT NO. 4402, UNIT I INGLEWOOD LINE B & C LOCATION MAP, GENERAL NOTES, INDEX TO DRAWINGS, STANDARD DRAWINGS, LEGEND AND ABBREVIATIONS.	
SUBMITTED BY: <i>R.K. Lee</i> ASSISTANT CITY ENGINEER R.C.E. 13078		RECOMMENDED BY: <i>Walter J. Moran</i> DIVISION ENGINEER (DESIGN)	
APPROVED BY: <i>Walter J. Moran</i> CITY ENGINEER R.C.E. 11237		APPROVAL RECOMMENDED BY: <i>Walter J. Moran</i> ASST. CHIEF DEPUTY ENGINEER	
SCALE AS SHOWN	DATE JAN. '66	DWG. NO. 364-4402-D2.1 SHEET 1 OF 4	DATE 12-2-65

"AS BUILT" DRAWING

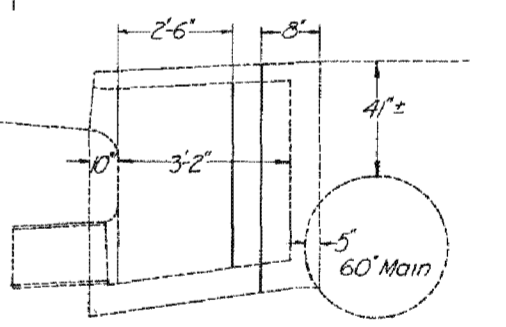


PROFILE
 SCALE: HORIZ. 1"=40'
 SCALE: VERT. 1"=4'

CROSS SECTION
 STA. 31+27
 SCALE: HORIZ. 1"=20'
 SCALE: VERT. 1"=4'



B.M. No 59 S.W. Cor. Prairie & Hardy L&PK in W. Curb of Prairie 3' S of B.C.R. E.I. 99.951



Detail for CB. No. 3 at Sta 26+45 and 29+75
 Width of catch basin inside shall be 2'-6" in lieu of 3'-2" as shown on standard plan 2-D-163

J.S. No 2 DATA

STA.	A	B	C	D	E	L	F	G	EL. "R"	EL. "S"
26+28	30"	18"	40"	39"	1.3	2.7	1.0	1.9	81.80	80.00
29+75	90"	18"	40"	36"	1.3	2.7	1.2	1.5	84.01	83.42

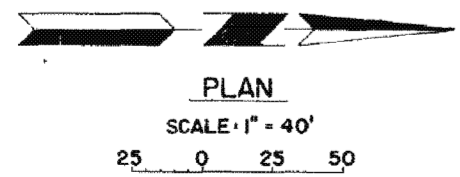
MH No 4 DATA

STA.	A	B	C	D	D ₂	F	EL. "R"	EL. "S"	
26+00	50"	18"	5"	39"	39"	7"	4 1/2"	80.37	79.86
29+75	90"	18"	4"	36"	39"	7"	4 1/2"	84.92	82.68
34+75	54"	24"	7"	36"	36"	6 1/2"	5 1/2"	88.37	87.49

NOTE CHANGE OF PROJECT LIMITS
 Due to bond issue financing requirements, the upstream limit of Project 681, has been changed to Station 14+98. See DWG. NO. 364-681-02.3. The downstream limit of Project 4402 Unit 1, Line C, has been extended to meet the new upstream limit of Project 681, see DWG. NO. 364-4402-D22.

REVISIONS

MARK	DATE	DESCRIPTION
5-4-67		Note Change Of Project Limits
7-26-68		"As Built"
11-22-72		PERMIT NO 69184 DWG. NO 364-4402-F5 PERMIT NO 69392
11-22-72		DWG. NO 364-4402-F6 11/6,9



1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD
 PUBLIC WORKS DEPARTMENT
 ENGINEERING DIVISION

LOS ANGELES COUNTY
 FLOOD CONTROL DISTRICT

PROJECT NO 4402, UNIT 1
 INGLEWOOD
 LINE C
 CONCRETE CONDUIT
 STA. 25+00 TO STA. 34+85
 PLAN AND PROFILE

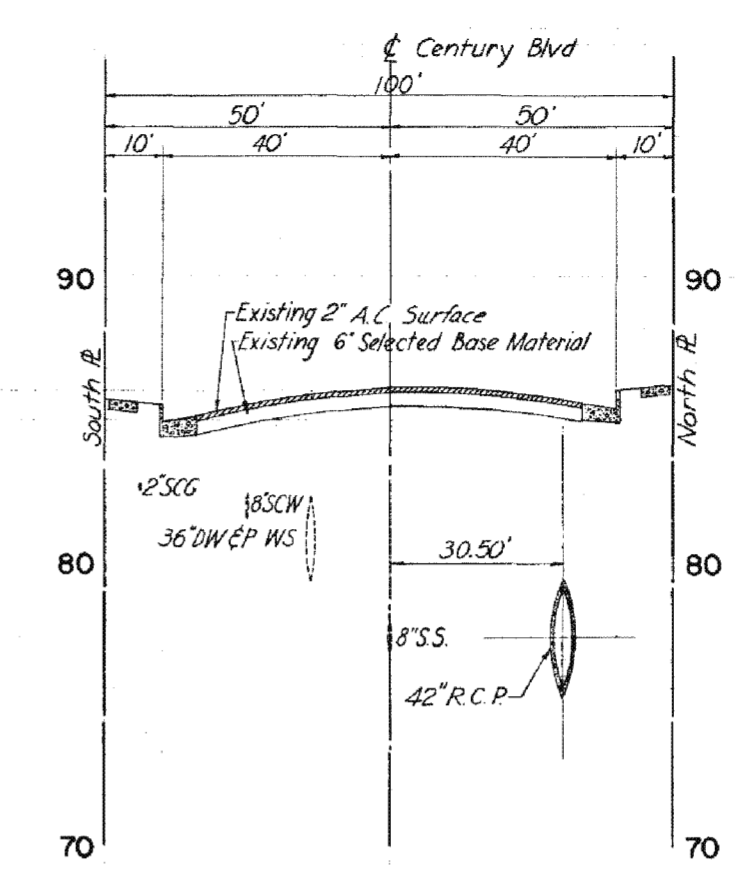
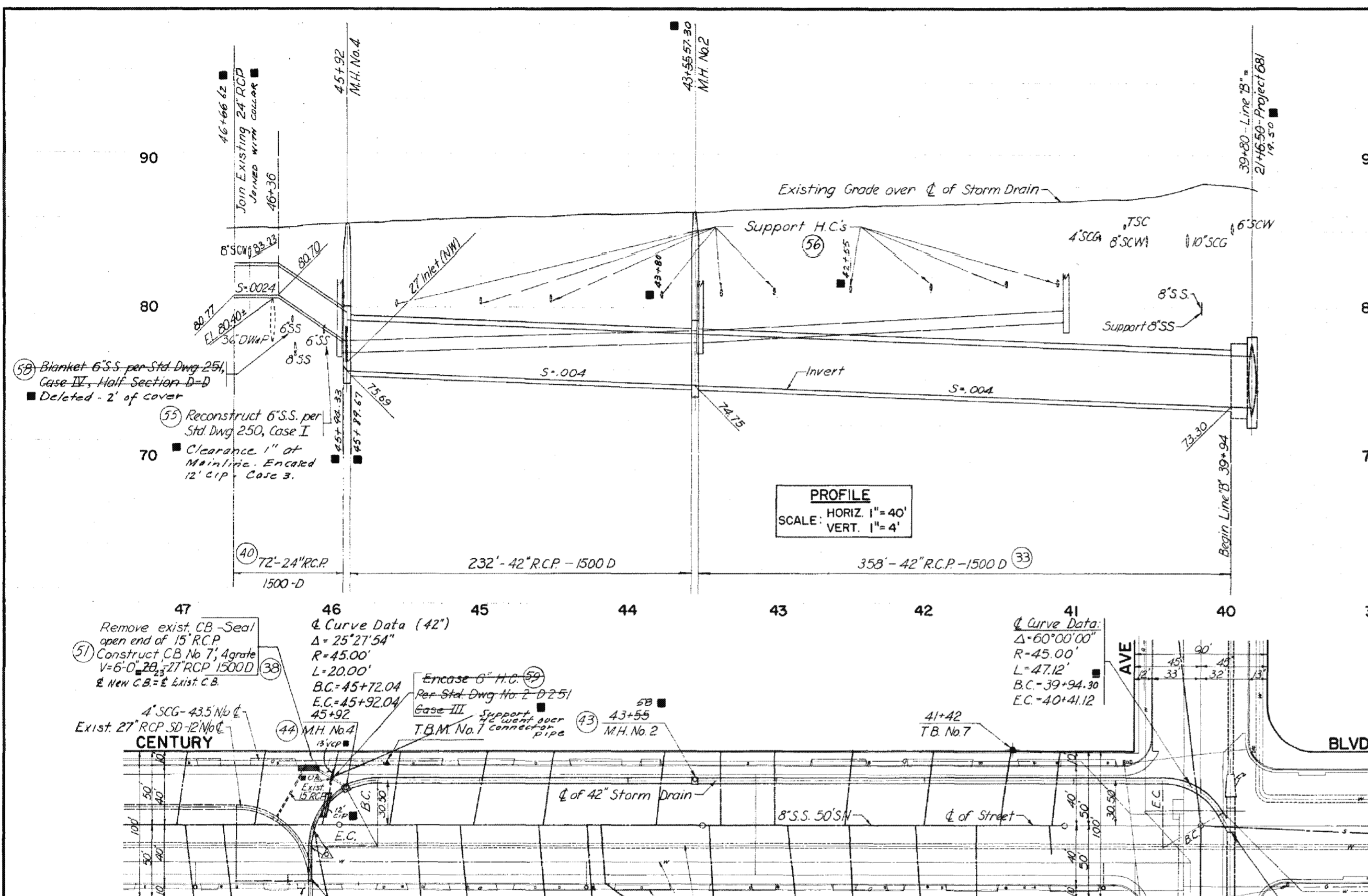
SUBMITTED BY:
 R. K. Lee
 ASSISTANT CITY ENGINEER R.C.E. 13078

APPROVED BY:
 W. J. Hamann
 CITY ENGINEER R.C.E. 11237

DATE: 11-15-65

SCALE: AS SHOWN
 DATE: JAN. 1966
 DWG. NO. 364-4402-D2.2
 SHEET 2 OF 4

"AS BUILT" DRAWING



PROFILE
 SCALE: HORIZ. 1" = 40'
 VERT. 1" = 4'

CROSS SECTION
 STA. 45+65
 SCALE: HORIZ. 1" = 20'
 VERT. 1" = 4'

T.B.M. No. 7 L&PK in N Curb of Century
 550' W. of center of Prairie El. 86.046

Remove exist. CB - Seal open end of 15" RCP
 Construct CB No. 7, 4 grate
 V=6'-0", 20' x 27" RCP 1500D
 & New CB = & Exist. CB

Curve Data (42")
 $\Delta = 25^\circ 21' 54''$
 $R = 45.00'$
 $L = 20.00'$
 $B.C. = 45+72.04$
 $E.C. = 45+92.04$
 45+92

Curve Data
 $\Delta = 60^\circ 00' 00''$
 $R = 45.00'$
 $L = 47.12'$
 $B.C. = 39+94.30$
 $E.C. = 40+41.12$

Curve Data (24")
 $\Delta = 64^\circ 32' 06''$
 $R = 45.00'$
 $L = 50.69'$
 $B.C. = 45+92.04$
 $E.C. = 46+42.73$

Construct 72" - 24" RCP 1500D
 Break out existing 24" RCP for connection and seal open end on west side of new conduit, Join 46+66

M.H. NO. 4 DATA STATION 45+92

A	B	C	D	D ₂	F	T	EL "R"	EL "S"
36"	27"	8"	24"	42"	7 1/2"	5 1/2"	76.27	75.68

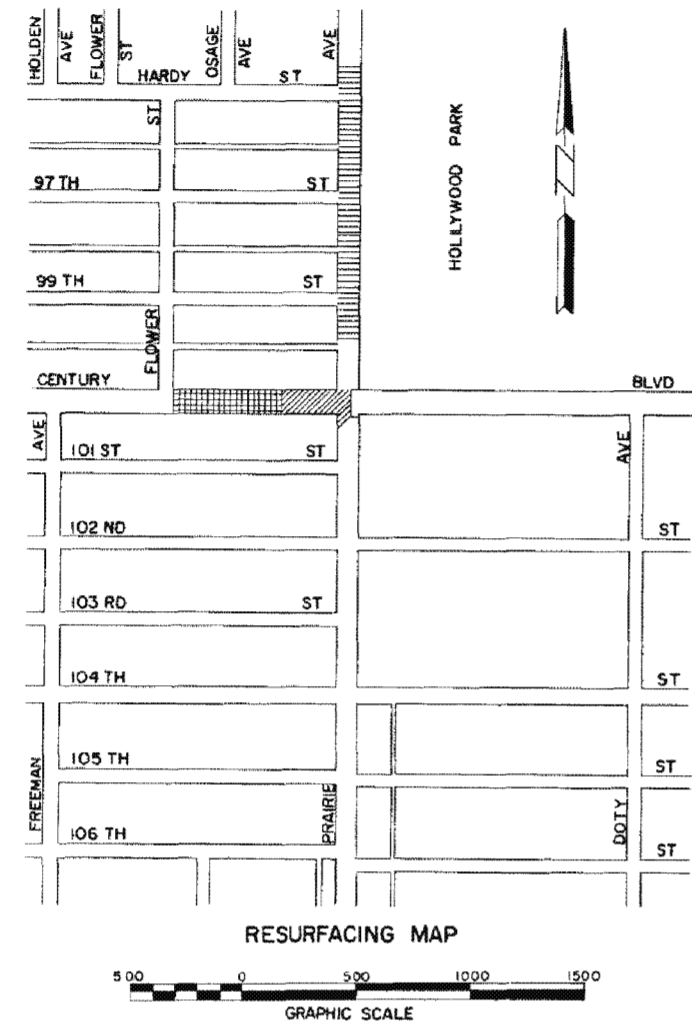
PLAN
 SCALE: 1" = 40'
 25 0 25 50

MARK	DATE	DESCRIPTION
■	7-26-68	"As Built"

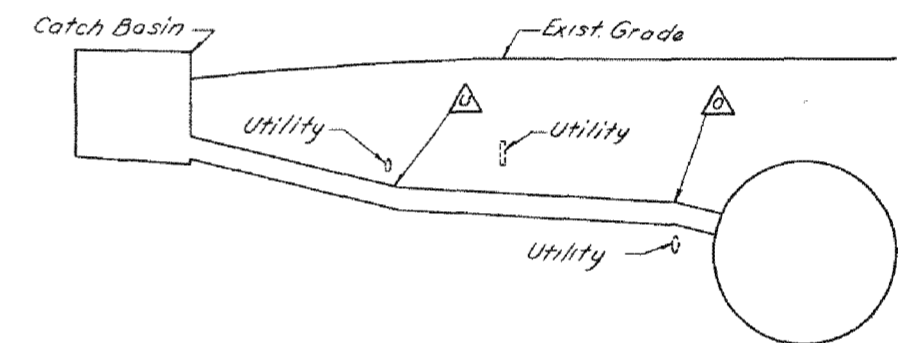
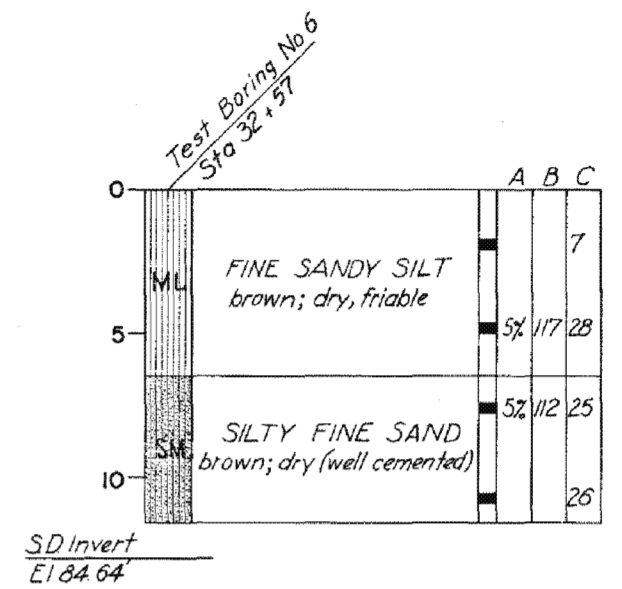
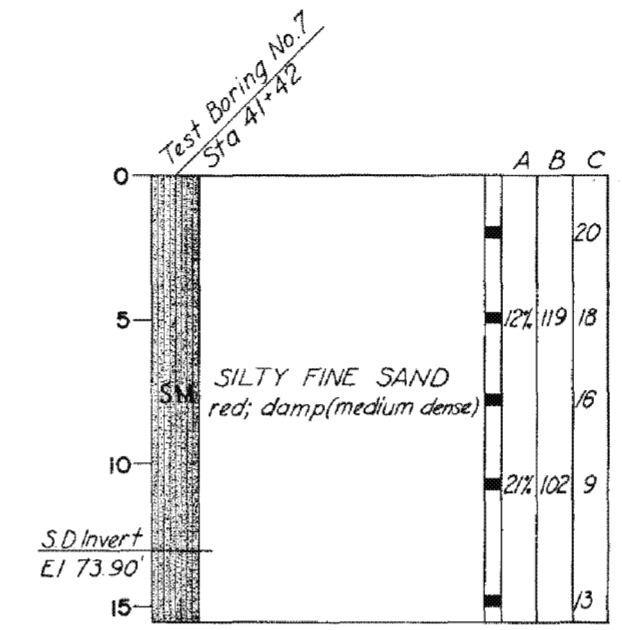
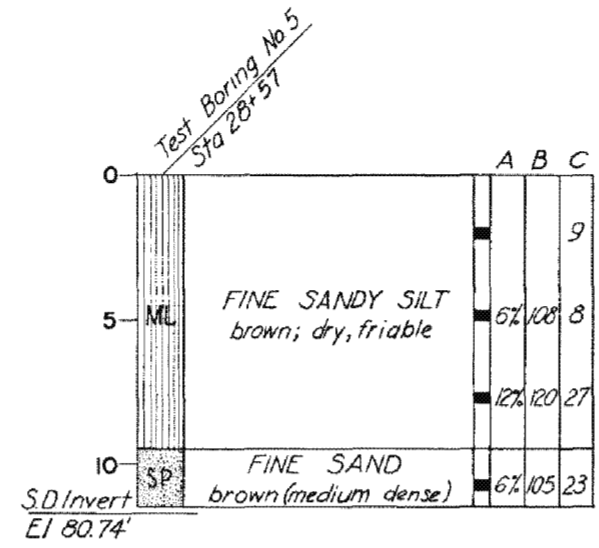
1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
	PROJECT NO. 4402, UNIT I INGLEWOOD LINE B CONCRETE CONDUIT STA. 39+80 TO STA. 46+66
SUBMITTED BY: <i>R.K. Yu</i> ASSISTANT CITY ENGINEER RCE 13078	PLAN AND PROFILE
APPROVED BY: <i>W. Haman</i> CITY ENGINEER RCE 11237	APPROVAL RECOMMENDING BY: <i>[Signature]</i> ASSISTANT CHIEF DEPUTY ENGINEER
DATE: 11-15-65	SCALE: AS SHOWN DATE: JAN. 1966 DWG. NO. 364-4402-D2.3 SHEET 3 OF 4

"AS BUILT" DRAWING



EXISTING SURFACE	PROPOSED RESURFACING
8" P.C.C. PAVEMENT ON 6" SELECTED BASE MATERIAL	8" P.C.C. PAVEMENT ON 6" SELECTED BASE MATERIAL (30)
2" A. C. SURFACE ON 6" SELECTED BASE MATERIAL	3" A. C. SURFACE ON 6" SELECTED BASE MATERIAL
2 1/2" A. C. SURFACE ON 6" PORTLAND CEMENT CONC.	2 1/2" A. C. SURFACE ON 6" PORTLAND CEMENT CONC.



LEGEND AND NOTES:

- SCALE: (VERTICAL) 1" = 4'
- A MOISTURE CONTENT IN PERCENTAGE
 - B DRY DENSITY IN LBS PER CUBIC FOOT
 - C STANDARD PENETRATION; NO. OF BLOWS PER FOOT
 - DEPTH AT WHICH UNDISTURBED SAMPLE WAS TAKEN
- ALL BORINGS WERE COMPLETED 8-26-65 AND 8-27-65.
 NO GROUND WATER OR CAVING WAS ENCOUNTERED IN ANY BORING.
 INVERT OF STORM DRAIN SHOWN ON THIS SHEET SUPERSEDES THAT SHOWN IN THE SOIL REPORT.
 THE SOIL CLASSIFICATIONS SHOWN ARE IN ACCORDANCE WITH THE UNIFIED SOIL CLASSIFICATION SYSTEM.

NOTES:

1. The change in grade of the connector pipe may occur either over or under an existing utility. The particular utility at which the change occurs is noted on the project drawings. At locations where utility crossings are marked 'o' the connector pipe grade will break over the utility. At locations where utility crossings are marked 'u' the connector pipe grade will break under the utility.
2. On those connector pipes where change in grade is not indicated, it is assumed that the connector pipe can be laid on a straight grade from the catch basin to the storm drain without interference with utilities.
3. Where connector pipe has a change of grade exceeding .10 ft or differs in diameter from that of existing pipe, use concrete collar per std 2-D 393.
4. The Contractor shall make exploratory excavation to determine the exact location and depth of utilities which are marked 'u' or 'o'. After the exact location of a utility has been determined, the grade and alignment of the connector pipe will be staked so as to clear the utility.

TYPICAL CONNECTOR PIPE PROFILE
NO SCALE

REVISIONS			
MARK	DATE	DESCRIPTION	



1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD
PUBLIC WORKS DEPARTMENT
ENGINEERING DIVISION

LOS ANGELES COUNTY
FLOOD CONTROL DISTRICT
PROJECT NO. 4402, UNIT I
INGLEWOOD
LOG OF BORINGS, RESURFACING PLAN AND
MISCELLANEOUS DETAILS

SUBMITTED BY:
R. K. Jew
ASSISTANT CITY ENGINEER R.C.E. 13078

APPROVED BY:
Walter J. Wood
CITY ENGINEER R.C.E. 11237

DATE: 12-2-65

RECOMMENDED BY: *W. J. Wood*
DIVISION ENGINEER (SENIOR)

APPROVAL RECOMMENDED BY: *W. J. Wood*
ASST. CHIEF DEPUTY ENGINEER

CHIEF ENGINEER

SCALE: AS SHOWN JAN 1966 DATE: 12-2-65 DWG. NO. 364-4402-02.4 SHEET 4 OF 4

LEGEND & ABBREVIATIONS

PROPERTY LINE	---
CURB	---
GUTTER	---
SIDE INLET CATCH BASIN	---
EXISTING DRAINAGE STRUCTURES	---
GRATING TYPE CATCH BASIN	---
DRIVEWAY	---
LIMIT OF CONCRETE SURFACE	---
WALK	---
TRAFFIC SIGNAL	---
DEAD MAN	---
POWER, TELEPHONE OR GUY POLE	---
FIRE HYDRANT	---
ELECTROLIER	---
WATER & GAS METER	---
BENCH MARK	---
TEST BORING	---
TREE	---
TREES TO BE REPLACED BY CONTRACTOR	---
UNDER EXISTING UTILITY	---
OVER EXISTING UTILITY	---
TRAFFIC SIGNAL CONDUIT	---
TRANSITION STRUCTURE	---
MANHOLE	---
LOCAL DEPRESSION	---
REINFORCED CONCRETE PIPE	---
TEST BORING	---
JUNCTION STRUCTURE	---
CATCH BASIN	---
SANITARY SEWER	---
TEMPORARY BENCH MARKS	---
SOUTHERN CALIFORNIA GAS CO.	---
L.A. DEPT. OF WATER & POWER WATER SYST. DW&PWS	---
SOUTHERN CALIFORNIA WATER CO.	---
INGLEWOOD CITY WATER DEPT.	---
SOUTHERN CALIFORNIA EDISON CO.	---
PACIFIC TELEPHONE & TELEGRAPH CO.	---

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

STANDARD DRAWINGS

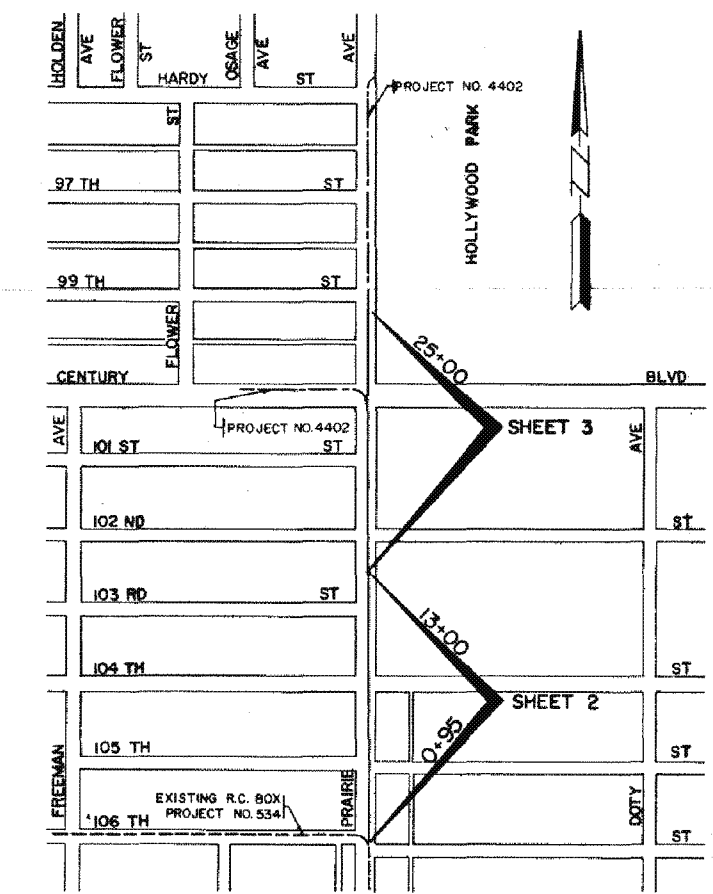
DRAWING NO.	TITLE
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2-D96	STANDARD DROP STEP
2-D107	CONCRETE RINGS, REDUCER AND PIPE
2-D193	JUNCTION STRUCTURE NO. 4
2-D156	MANHOLE FRAME AND COVER FOR CATCH BASINS
2-D160	CATCH BASIN NO. 1 (W=3'6")
2-D162	CATCH BASIN NO. 2 (W=7')
2-D163	CATCH BASIN NO. 3 (W=10'; 14'; 21'; 28'; ETC.)
2-D172	CATCH BASIN REINFORCEMENT
2-D173.1 TO 3	PIPE SUPPORTS ACROSS TRENCHES
2-D175	REMOVABLE PROTECTION BAR FOR CATCH BASINS
2-D177	PIPE BEDDING IN TRENCHES
2-D181	STANDARD NON-ROCKING MANHOLE FRAME AND COVER
2-D184	MANHOLE NO. 2 (PIPES, 36" OR LARGER)
2-D188	TRANSITION STRUCTURE NO. 3 (PIPE TO PIPE)
2-D213.1 & 2	"D" LOAD TABLE FOR DESIGN OF REINFORCED CONCRETE PIPE
2-D171	STANDARD A-305 REINFORCING BARS
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2-D224	CONNECTION TO CATCH BASIN FOR PIPES 12" THROUGH 72"
2-D227	FRAME AND GRATING FOR CATCH BASINS
2-D249.1 & 2	CATCH BASIN NO. 8
2-D264	ADJUSTABLE PROTECTION BAR STIRRUP
2-D393	CONCRETE COLLAR FOR PIPES 12" THROUGH 66"
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2-D399	CRITERIA FOR THE DESIGN OF SHORING FOR EXCAVATIONS
2-D400	SAMPLE SHEET FOR USE AS A GUIDE IN PREPARING CALCULATIONS FOR SHORING OF EXCAVATIONS
2-D113	MANHOLE NO. 4
2-D112	JUNCTION STRUCTURE NO. 2
2-D161	DETAIL OF ANGLE AND ANCHOR FOR CATCH BASINS

GENERAL NOTES

- NUMBERS IN CIRCLES INDICATE ITEMS UNDER WHICH PAYMENT WILL BE MADE.
- ELEVATIONS SHOWN ARE IN FEET ABOVE THE U.S.G.S. MEAN SEA LEVEL DATUM.
- STATIONS SHOWN ON DRAWINGS ARE ALONG CENTER LINE OF CONDUIT OR ON A LINE NORMAL TO CENTER LINE OF CONDUIT.
- STATIONS AND INVERT ELEVATIONS OF PIPE INLETS SHOWN ON THE PROFILES ARE AT THE INSIDE FACE OF THE CONDUIT, UNLESS OTHERWISE SHOWN.
- ALL FIELD BOOK REFERENCES ARE TO CITY OF INGLEWOOD FIELD BOOKS, UNLESS OTHERWISE NOTED.
- ALL PIPE IN OPEN TRENCH SHALL BE BEDDED ACCORDING TO STANDARD DRAWING 2-D 177, CASE III, UNLESS OTHERWISE SHOWN OR MODIFIED IN THE SPECIFICATIONS.
- PIPE CONNECTIONS TO STORM DRAIN SHALL CONFORM TO STANDARD DRAWING 2-D 193, UNLESS OTHERWISE SHOWN.
- TIES FOR CATCH BASINS AS SHOWN ON THE DRAWINGS ARE FROM CURB RETURN TO CENTER LINE OF CATCH BASIN, UNLESS OTHERWISE SHOWN.
- LOCATIONS OF CATCH BASIN CONNECTOR PIPE JUNCTIONS WITH CATCH BASINS AS SHOWN ON THE DRAWINGS ARE SCHEMATIC. IT IS INTENDED THAT SUCH JUNCTIONS BE LOCATED AT THE DOWNSTREAM ENDS OF THE CATCH BASINS, UNLESS OTHERWISE SHOWN. IN ALL CASES THE EXACT LOCATIONS WILL BE DETERMINED IN THE FIELD BY THE ENGINEER TO MEET FIELD CONDITIONS.
- MONOLITHIC CATCH BASIN CONNECTIONS SHALL BE CONSTRUCTED, WHERE APPLICABLE, PER STANDARD DRAWING 2-D 224.
- "V_i" IS THE DEPTH OF INLET OF CATCH BASINS IN SERIES MEASURED FROM TOP OF CURB TO INVERT OF CONNECTOR PIPE.
- ALL EXISTING SANITARY SEWERS SHOWN ON THE DRAWINGS ARE CITY OF INGLEWOOD SEWERS.
- EXISTING UTILITIES SHALL BE MAINTAINED IN PLACE BY THE CONTRACTOR, UNLESS OTHERWISE NOTED.
- WHERE UTILITIES ARE INDICATED ON THE DRAWINGS TO BE SUPPORTED, SAID SUPPORTS SHALL BE IN ACCORDANCE WITH STANDARD DRAWING 2-D 173.1, .2 OR .3, UNLESS OTHERWISE INDICATED.
- LOCATIONS SHOWN ON THE PLANS FOR EXISTING SANITARY SEWER HOUSE CONNECTIONS ARE APPROXIMATE ONLY.
- SANITARY SEWER HOUSE CONNECTION RECONSTRUCTION AND RECONNECTION SHALL BE IN ACCORDANCE WITH STANDARD DRAWING 2-D 250, UNLESS OTHERWISE SHOWN.
- SANITARY SEWERS AND HOUSE CONNECTIONS CROSSING OVER THE STORM DRAIN TRENCH SHALL BE SUPPORTED IN ACCORDANCE WITH STANDARD DRAWINGS 2-D 173.1 TO .3 AND ENCASED PER GENERAL NOTE 1 ON STANDARD DRAWING 2-D 173.1.
- WHEN INDICATED ON THE DRAWINGS, SANITARY SEWERS AND HOUSE CONNECTIONS SHALL BE ENCASED OR BLANKETED IN ACCORDANCE WITH STANDARD DRAWING 2-D 251.
- ALL RESURFACING, CURBS, GUTTERS, SIDEWALKS, DRIVEWAYS AND OTHER EXISTING IMPROVEMENTS TO BE RECONSTRUCTED SHALL BE CONSTRUCTED AT THE SAME ELEVATION AND LOCATION AS THE EXISTING IMPROVEMENTS, UNLESS OTHERWISE NOTED.
- SOIL TEST BORINGS FOR THIS PROJECT WERE MADE 8-26-65.
- UTILITIES DESIGNATED BY THE SYMBOL * WILL BE ABANDONED IN PLACE AND THE OWNER WILL INSTALL A NEW SECTION OF THE AFFECTED UTILITY AT A LOCATION IN CLOSE PROXIMITY TO, BUT WHICH DOES NOT PHYSICALLY INTERFERE WITH, THE PROPOSED STORM DRAIN CONDUIT AND APPURTENANT STRUCTURES.
- THE DEPTH AT THE UPSTREAM END OF CATCH BASINS 10 FEET OR MORE IN LENGTH SHALL BE CURB FACE PLUS 12 INCHES UNLESS OTHERWISE SHOWN.
- REFER TO SHEET 4 FOR TYPICAL CATCH BASIN CONNECTOR PIPE PROFILE.
- ALL OPENINGS RESULTING FROM THE CUTTING OR PARTIAL REMOVAL OF EXISTING PIPES, CULVERTS, OR SIMILAR STRUCTURES SHALL BE SEALED WITH 8 INCHES OF BRICK AND MORTAR OR 6 INCHES OF CONCRETE.
- THE OPENINGS AND TOP SLABS OF ALL SIDE INLET CATCH BASINS AND CATCH BASIN NO. 8 SHALL BE MODIFIED TO MEET THE REQUIREMENTS ON STANDARD DRAWING 2-D232 EXCEPT AS NOTED.
- WHERE REQUIRED BY STANDARD DRAWING 2-D 213.1, CONCRETE BACKFILL SHALL BE USED AROUND CONNECTOR PIPES 36 INCHES OR LESS IN DIAMETER. CONCRETE BACKFILL FOR MAIN LINE PIPE SHALL BE USED ONLY WHEN DIRECTED BY THE ENGINEER.
- THE OPENING OF ALL SIDE INLET CATCH BASINS AND CATCH BASIN NO. 6 THAT HAVE MODIFIED LOCAL DEPRESSIONS TO BE CURB FACE PLUS 1" IN LIEU OF 9"

INDEX TO DRAWINGS

SHEET NO.	TITLE
1.	LOCATION MAP, GENERAL NOTES, INDEX TO DRAWINGS, STANDARD DRAWINGS, ABBREVIATIONS AND LEGEND.
2.	PLAN, PROFILE AND SECTION STA 0+95 TO STA 13+00
3.	PLAN, PROFILE AND SECTION STA 13+00 TO STA 25+00
4.	LOG OF BORINGS & RESURFACING PLAN



NOTE CHANGE OF PROJECT LIMITS
Due to bond issue financing requirements, the upstream limit of Project 681, has been changed to Station 14+98. See DWG. NO. 364-681-D2.3. The downstream limit of Project 4402 Unit I, Line C, has been extended to meet the new upstream limit of Project 681, see DWG. NO. 364-4402-D2.2

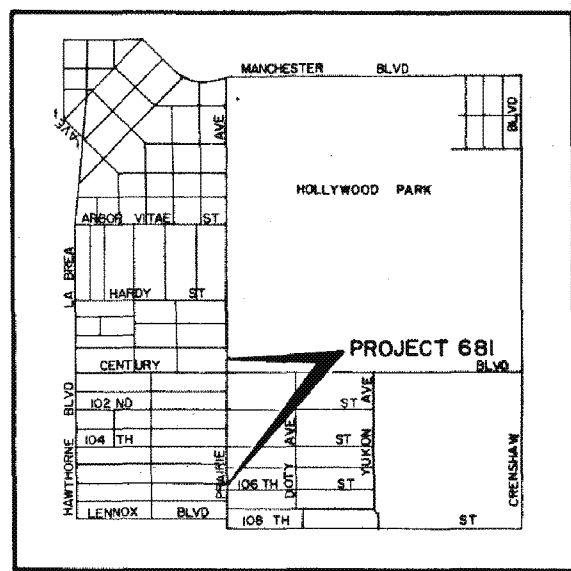
LOS ANGELES COUNTY ROAD DEPARTMENT

STANDARD DRAWINGS

DRAWING NO.	TITLE
M57-39R	PARTIAL CONCRETE REPLACEMENT FOR CROSS GUTTERS AND SPANDRELS.
M57-45R	SPECIFICATIONS FOR REPAIRS OF CUTS IN CONCRETE PAVEMENT.

- CONSTRUCT LOCAL DEPRESSION PER STANDARD DRAWING 2-D109 AND AS MODIFIED ON THE DRAWINGS FOR CATCH BASIN NO. 6.

"AS BUILT DWGS."



VICINITY MAP
1000 0 2000 3000 4000
GRAPHIC SCALE

DATE	REVISIONS
4-24-67	Note Change Of Project Limits

1964 STORM DRAIN BOND ISSUE

CITY OF INGLEWOOD
PUBLIC WORKS DEPARTMENT
ENGINEERING DIVISION

LOS ANGELES COUNTY
FLOOD CONTROL DISTRICT
PROJECT NO. 681
PRAIRIE AVENUE

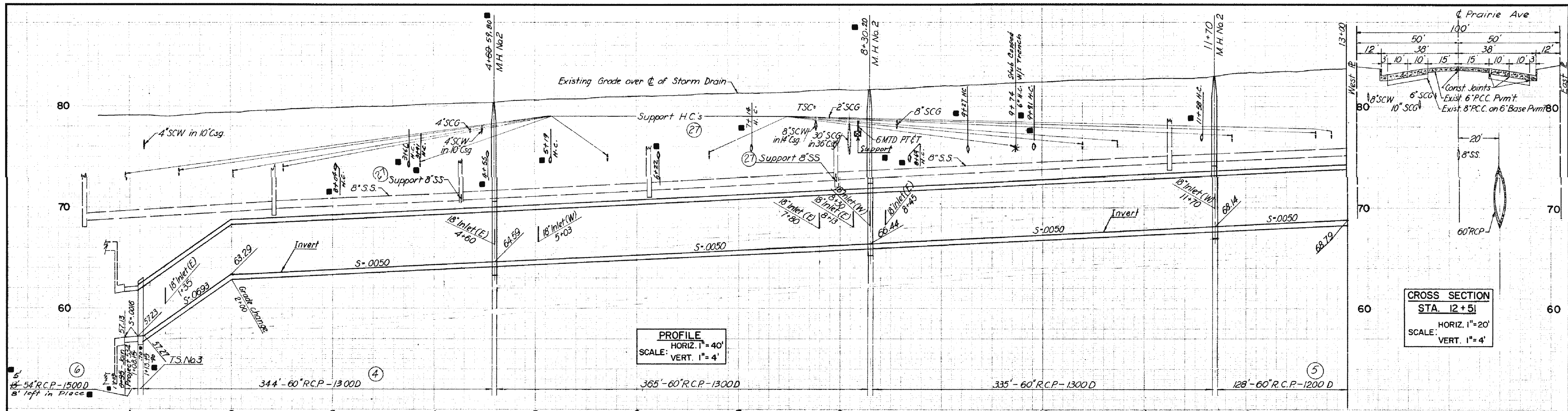
LOCATION MAP, GENERAL NOTES, INDEX TO DRAWINGS, STANDARD DRAWINGS, LEGEND AND ABBREVIATIONS.

SUBMITTED BY: *R.K. Lee*
ASSISTANT CITY ENGINEER R.C.E. 13078

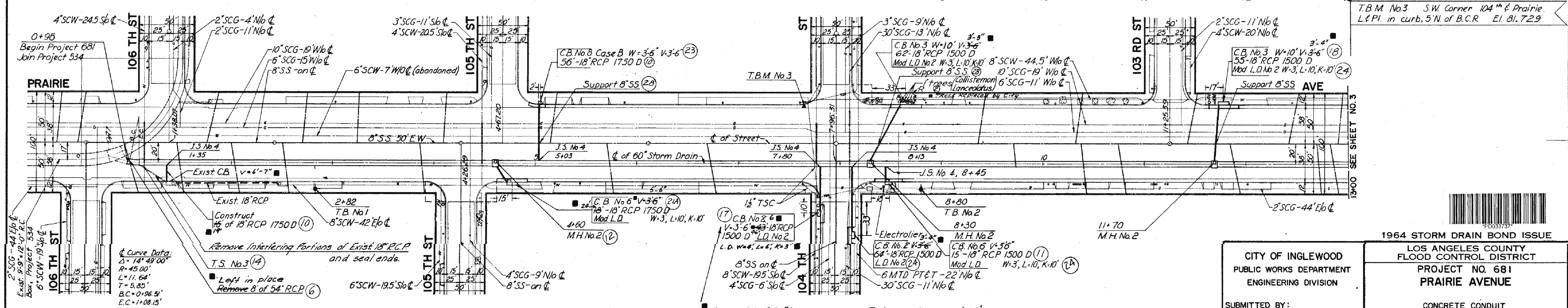
APPROVED BY: *Walter J. Hanson* 11-15-65
CITY ENGINEER R.C.E. 11237

SCALE: AS SHOWN
DATE: JAN. 11, 1966
DWG. NO. 364-681-D2.1
SHEET 1 OF 4

"AS BUILT" DRAWING



CROSS SECTION
 STA. 12+51
 SCALE: HORIZ. 1"=20'
 VERT. 1"=4'



MARK	DATE	DESCRIPTION
1	7-18-68	"As Built"

PLAN
 SCALE: 1" = 40'
 25 0 25 50

Changed to C.B. #6 because of interfering water line valve, fire hydrant & traffic signal.

Conn. pipe moved 1.5' north to pass an E.T.S. of 30" S.C.B.

1964 STORM DRAIN BOND ISSUE

LOS ANGELES COUNTY
 FLOOD CONTROL DISTRICT

PROJECT NO. 681
PRAIRIE AVENUE

CITY OF INGLEWOOD
 PUBLIC WORKS DEPARTMENT
 ENGINEERING DIVISION

SUBMITTED BY:
 R.K. Lee
 ASSISTANT CITY ENGINEER R.C.E. 13078

APPROVED BY:
 [Signature]
 CITY ENGINEER R.C.E. 11237

CONCRETE CONDUIT
 TO STA. 13+00

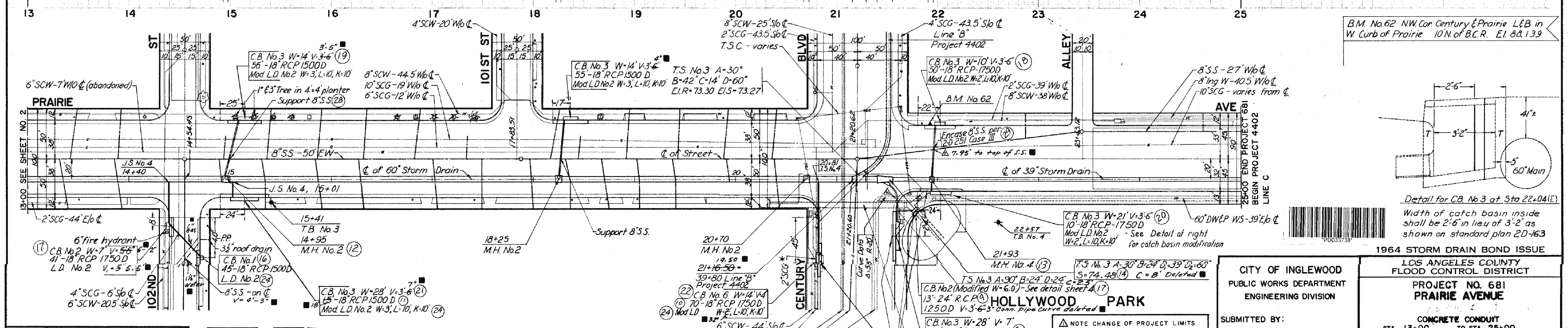
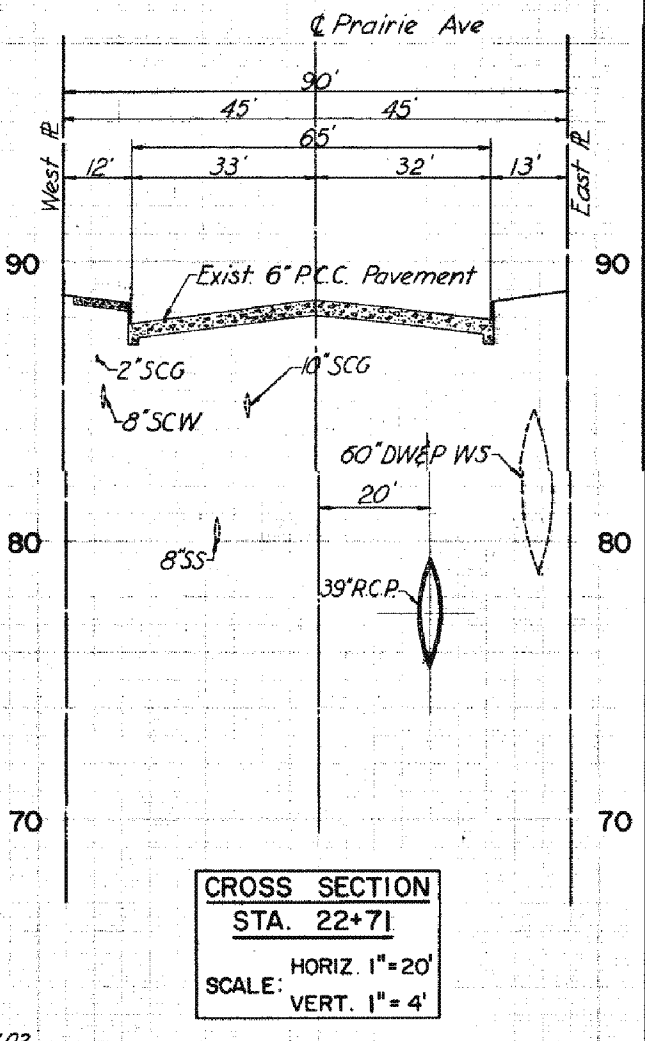
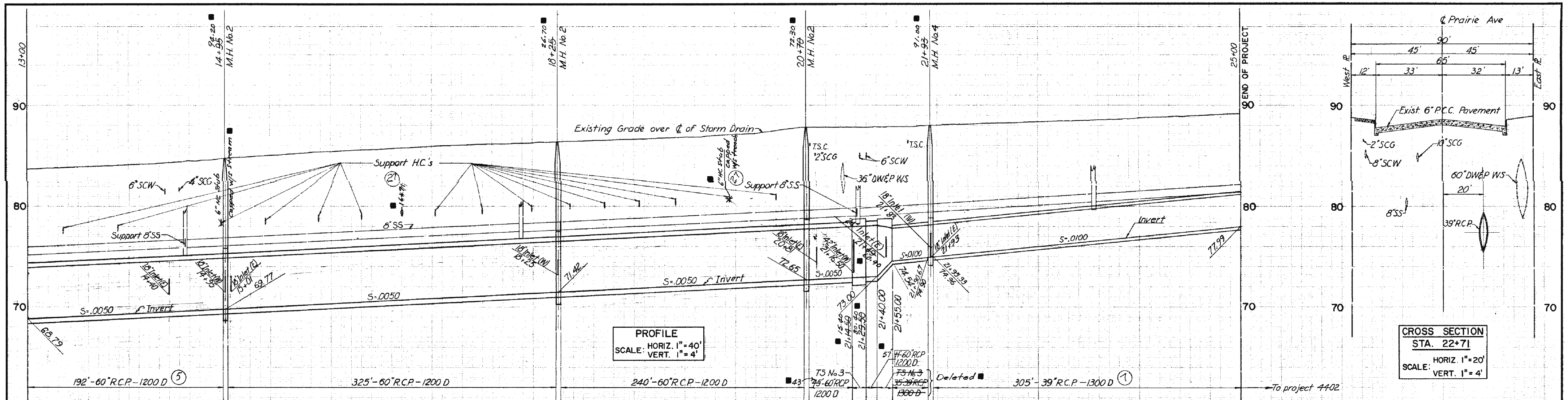
PLAN AND PROFILE

RECOMMENDED BY:
 [Signature]
 DIVISION ENGINEER

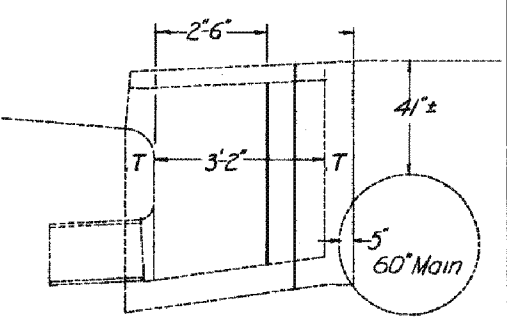
APPROVAL RECOMMENDED BY:
 [Signature]
 ASST. CHIEF DEPUTY ENGINEER

SCALE: AS SHOWN
 DATE: JAN. 13 1966
 DWG. NO. 364-681-D-2
 SHEET 2 OF 4

"AS BUILT" DRAWING



B.M. No. 62 NW. Cor Century & Prairie L&B in W. Curb of Prairie 10' N of B.C.R. E.I. 80.139

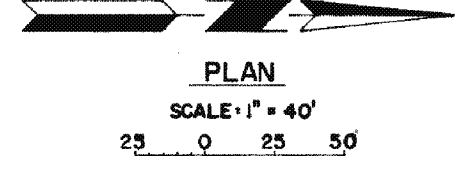


Detail for CB No. 3 at Sta 22+04.1E
Width of catch basin inside shall be 2'-6" in lieu of 3'-2" as shown on standard plan 2D-163



1964 STORM DRAIN BOND ISSUE

MARK	DATE	DESCRIPTION
Δ	4-25-67	Note Change Of Project Limits
■	7-19-68	"As Built"



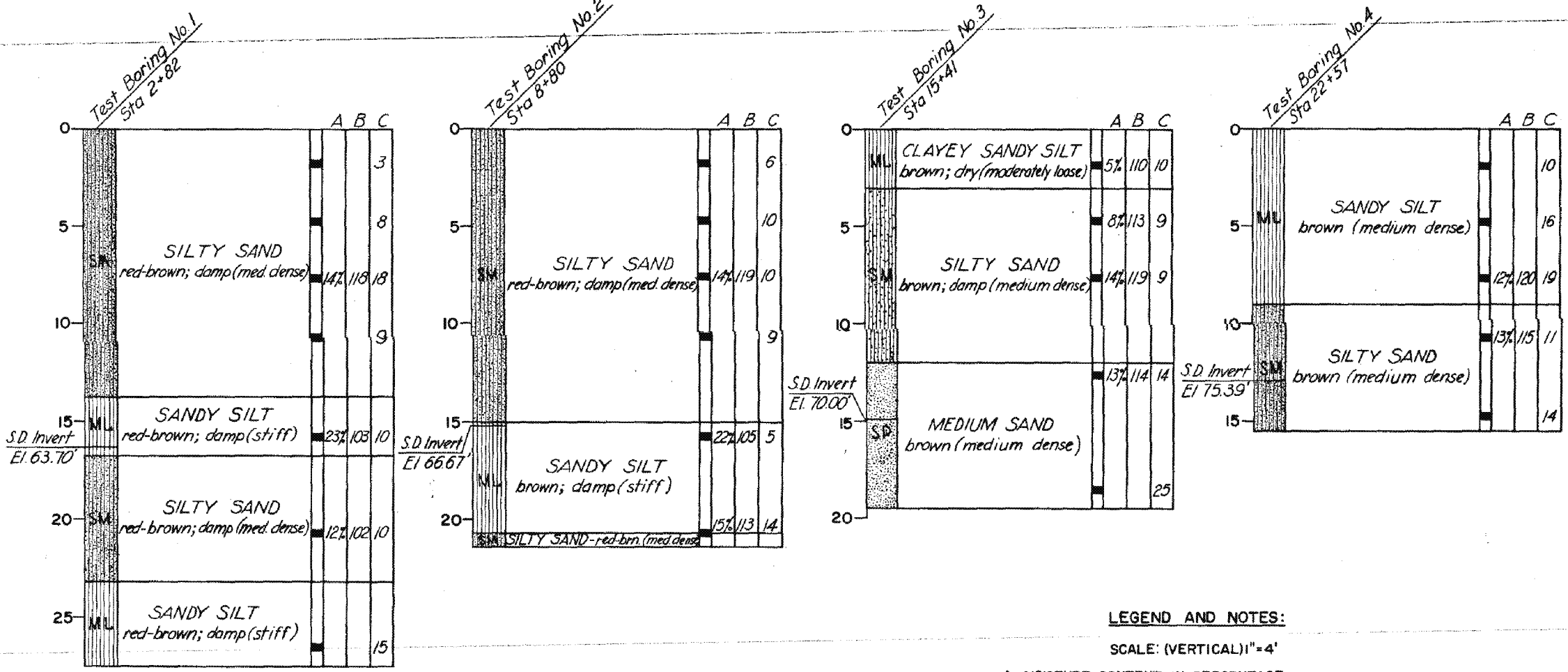
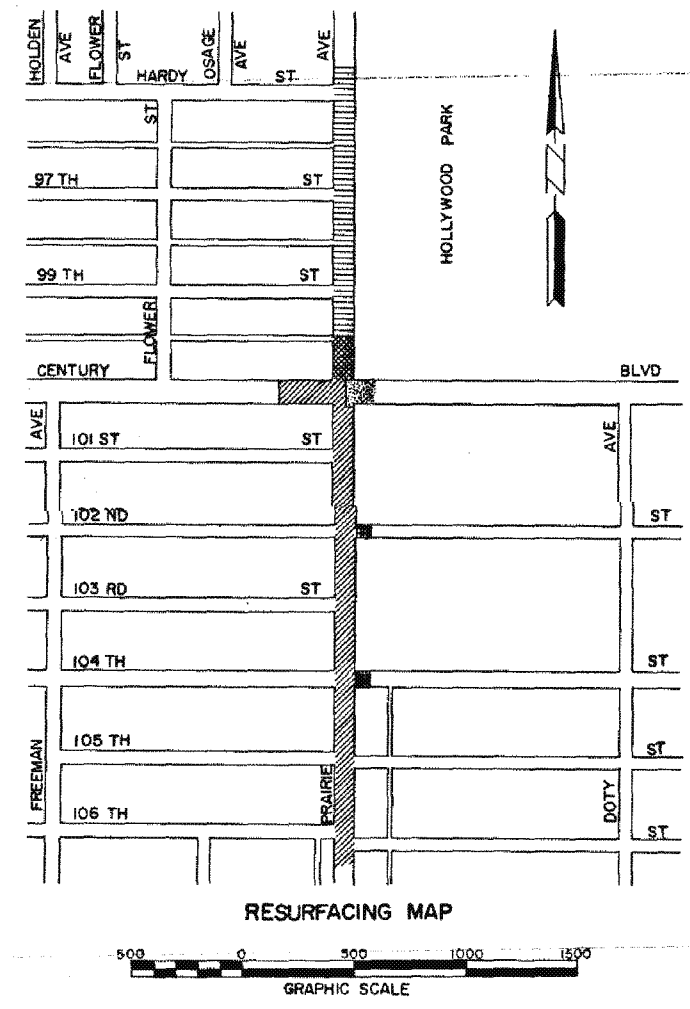
M.H. No. 4 DATA - STA. 21+93

INLET	A	B	C	D	E	F	T	EL. 70"	EL. 75"
EAST	20"	18"	40"	39"	39"	7"	4"	70.26	70.92
WEST	06"	18"	20"					76.33	75.79

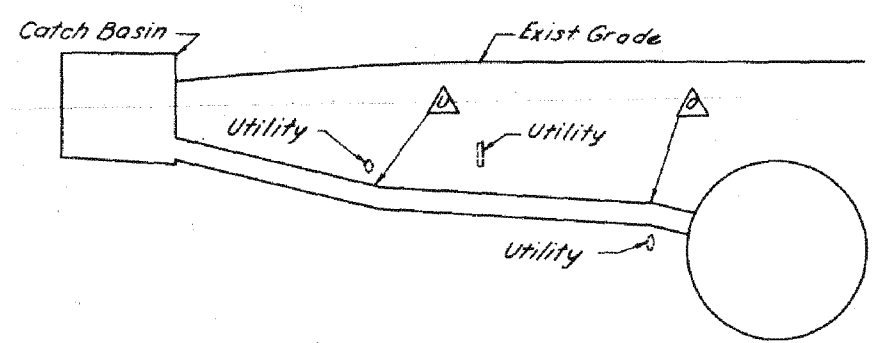
NOTE CHANGE OF PROJECT LIMITS
Due to bond issue financing requirements, the upstream limit of Project 681, has been changed to Station 14+98. See DWG NO. 364-681-D23. The downstream limit of Project 4402 Unit I, Line C, has been extended to meet the new upstream limit of Project 681, see DWG NO. 364-4402-D22.

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: R. K. [Signature] ASSISTANT CITY ENGINEER R.C.E. 13078		PROJECT NO. 681 PRAIRIE AVENUE CONCRETE CONDUIT STA. 13+00 TO STA. 25+00 PLAN AND PROFILE	
APPROVED BY: W. [Signature] CITY ENGINEER R.C.E. 11237		APPROVED BY: [Signature] CHIEF ENGINEER	
SCALE	DATE	DWG. NO.	SHEET
AS SHOWN	JAN 1966	364-681-D 2.3	3 OF 4

"AS BUILT" DRAWING



TYPICAL CONNECTOR PIPE PROFILE
NO SCALE

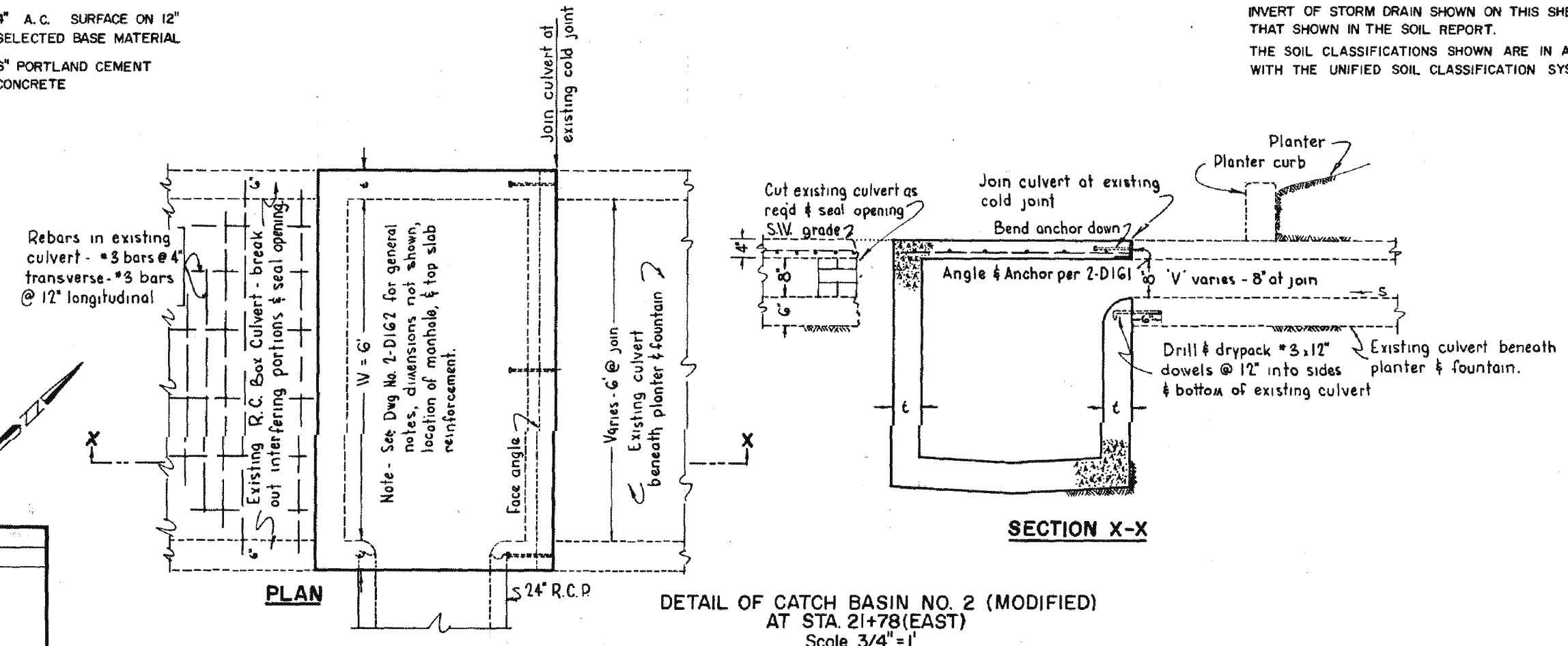


EXISTING SURFACE	PROPOSED RESURFACING
8" P.C.C. PAVEMENT ON 6" SELECTED BASE MATERIAL	8" P.C.C. PAVEMENT ON 6" SELECTED BASE MATERIAL
2 1/2" A.C. SURFACE ON 6" PORTLAND CEMENT CONC.	2 1/2" A.C. SURFACE ON 6" PORTLAND CEMENT CONC.
3" A.C. SURFACE ON 12" SELECTED BASE MATERIAL	4" A.C. SURFACE ON 12" SELECTED BASE MATERIAL
6" PORTLAND CEMENT CONCRETE	6" PORTLAND CEMENT CONCRETE

LEGEND AND NOTES:

- SCALE: (VERTICAL) 1"=4'
- A MOISTURE CONTENT IN PERCENTAGE
 - B DRY DENSITY IN LBS PER CUBIC FOOT
 - C STANDARD PENETRATION; NO. OF BLOWS PER FOOT
 - DEPTH AT WHICH UNDISTURBED SAMPLE WAS TAKEN
- ALL BORINGS WERE COMPLETED 8-26-65.
NO GROUND WATER OR CAVING WAS ENCOUNTERED IN ANY BORING.
INVERT OF STORM DRAIN SHOWN ON THIS SHEET SUPERSEDES THAT SHOWN IN THE SOIL REPORT.
THE SOIL CLASSIFICATIONS SHOWN ARE IN ACCORDANCE WITH THE UNIFIED SOIL CLASSIFICATION SYSTEM.

- NOTES:
- The change in grade of the connector pipe may occur either over or under an existing utility. The particular utility at which the change occurs is noted on the project drawings. At locations where utility crossings are marked Δ , the connector pipe grade will break over the utility. At locations where utility crossings are marked ∇ , the connector pipe grade will break under the utility.
 - On those connector pipes where change in grade is not indicated, it is assumed that the connector pipe can be laid on a straight grade from the catch basin to the storm drain without interference with utilities.
 - Where connector pipe has a change of grade exceeding 10'/ft or differs in diameter from that of existing pipe, use concrete collar per std 2-D393.
 - The contractor shall make exploratory excavations to determine the exact location and depth of utilities, which are marked "A" or "B". After the exact location of a utility has been determined, the grade and alignment of the connector pipe will be staked so as to clear the utility.



MARK	DATE	DESCRIPTION
1	7-19-68	"AS BUILT"

CITY OF INGLEWOOD PUBLIC WORKS DEPARTMENT ENGINEERING DIVISION		LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
SUBMITTED BY: R.K. [Signature] ASSISTANT CITY ENGINEER R.C.E. 13078		PROJECT NO. 681 PRAIRIE AVENUE	
APPROVED BY: [Signature] CITY ENGINEER R.C.E. 11237		LOG OF BORINGS, RESURFACING PLAN AND MISCELLANEOUS DETAILS	
SCALE AS SHOWN	DATE JAN. 12 '66	DWG. NO. 364-681-D2.4	SHEET 4 OF 4

"AS BUILT" DRAWING